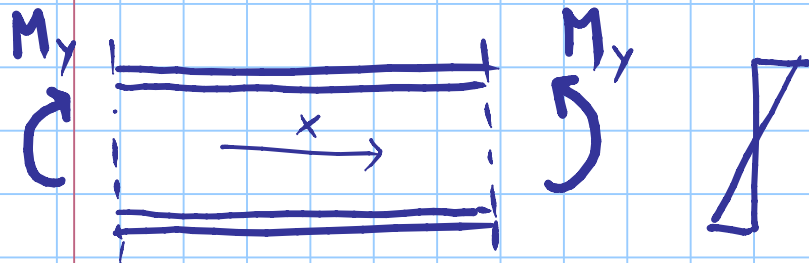


# FLESSIONE SEMPLICE RETTA

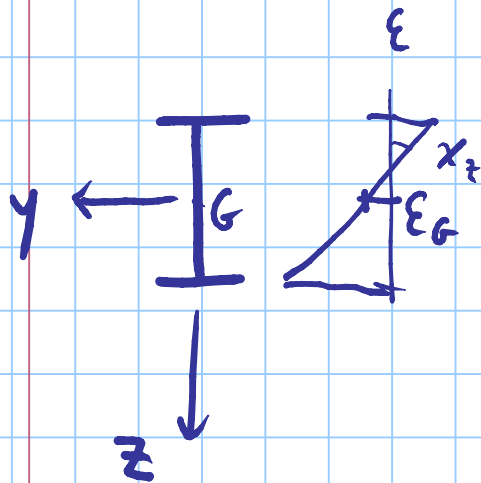
Titolo nota

12/11/2013



comportamento  
in campo elastico

$$\sigma = E \varepsilon$$



mantenimento  
sezione piana

$$\varepsilon = \varepsilon_G + \chi_z z$$

$$\chi_z = \frac{1}{\rho} = \frac{d\varepsilon}{dz}$$

$$N = \int \sigma dA = \int E(\epsilon_c + \chi_z z) dA =$$

$$= E \epsilon_c \underbrace{\int dA}_A + E \chi_z \underbrace{\int z dA}_{S_y = 0} = E \epsilon_c A$$

$$\epsilon_c = \frac{N}{EA}$$

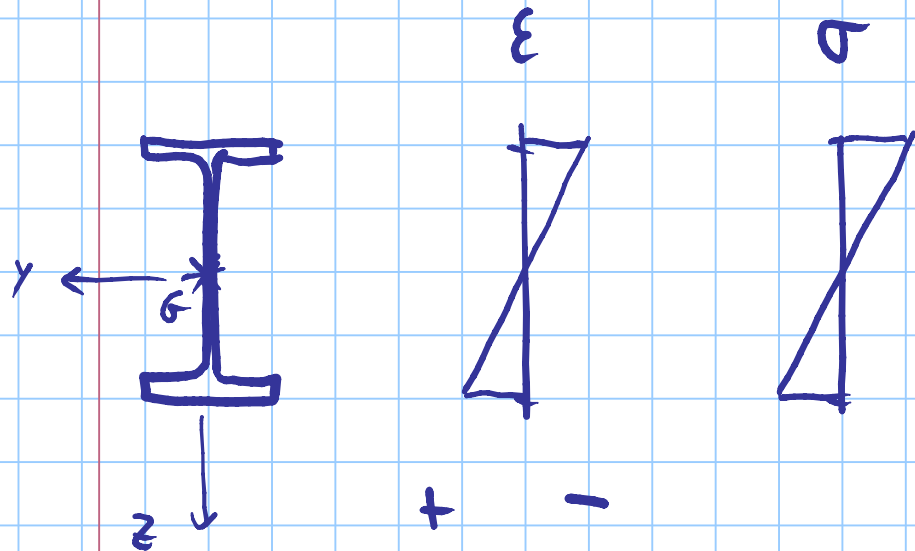
flexion simple  $N=0 \Rightarrow \epsilon_c = 0$

$$M_y = \int \sigma z \, dA = \int E(\epsilon_c + \chi_z z) z \, dA =$$

$$= E \epsilon_c \underbrace{\int z \, dA}_{S_y = 0} + E \chi_z \underbrace{\int z^2 \, dA}_{I_y} = E \chi_z I_y$$

$$\chi_z = \frac{M_y}{E I_y}$$

$$\sigma = \underbrace{\epsilon_c}_0 + \underbrace{\chi_z}_{\frac{M}{EI}} z = \frac{M_y}{E I_y} z$$



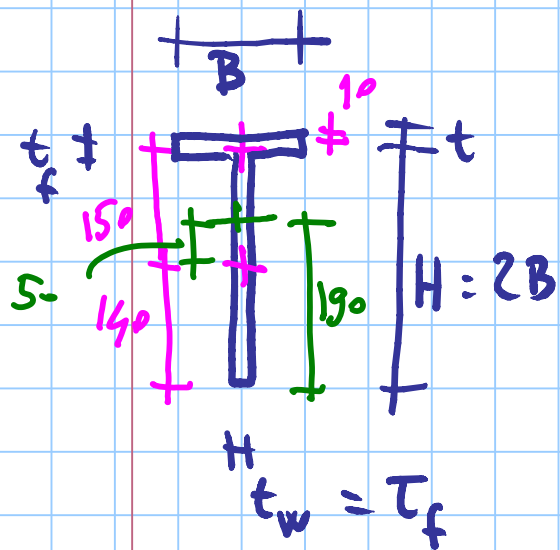
$$\sigma = \frac{M}{I} z \approx \sigma$$

$$|\sigma_{\max}| = \frac{M}{I} |z_{\max}|$$

$$\frac{I}{|z_{\max}|} = W \quad \text{modulo di resistenza}$$

$$\sigma_{\max} = \frac{M}{W}$$

$$t = 20 \text{ mm}$$



$$H = 280 \text{ mm}$$

$$B = 140 \text{ mm}$$

## ANDANDO OLTRE IL LIMITE ELASTICO

comportamento  
in campo elastico

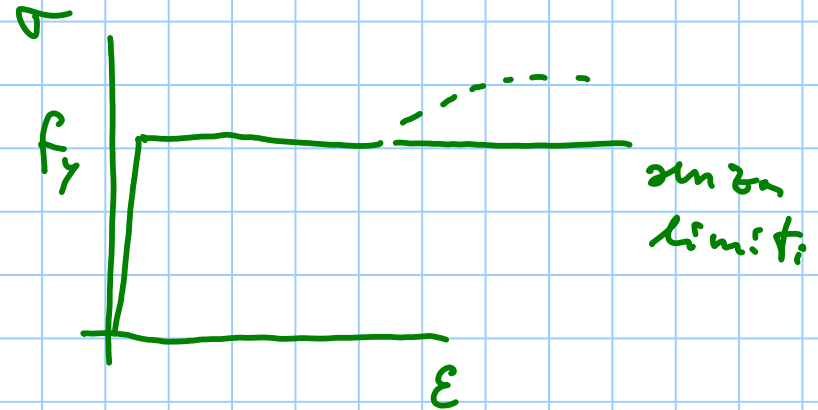
$$\sigma = E \varepsilon$$

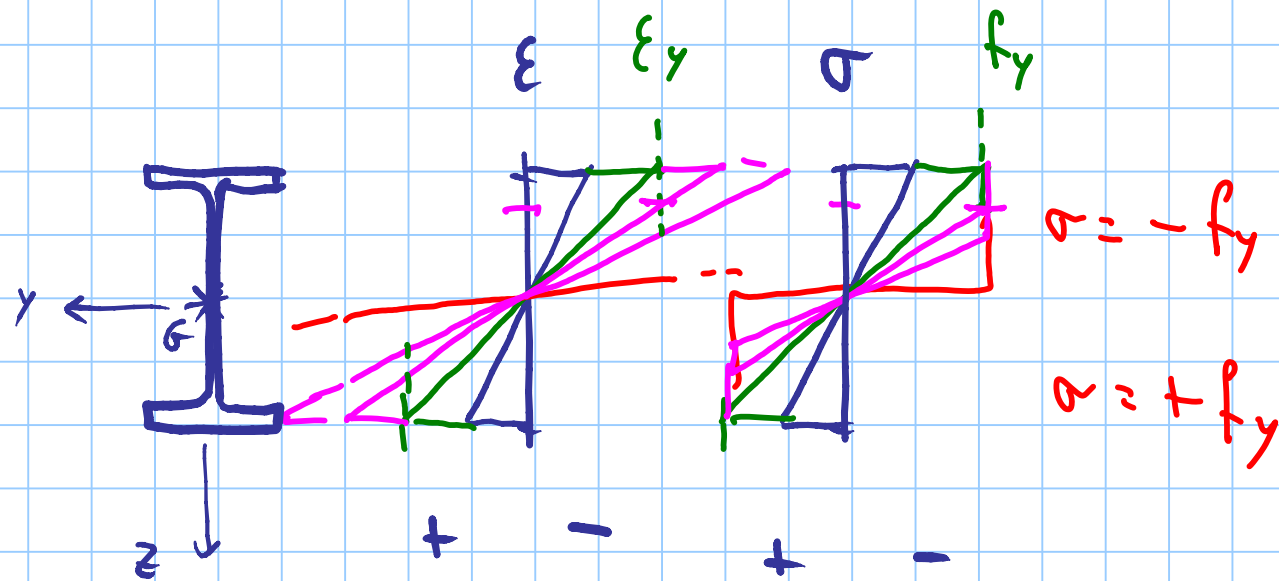
No

mantenimento  
sezione piana

$$\varepsilon = \varepsilon_f + \chi_z z$$

SI





$$\sigma = \frac{M}{W} = f_y$$

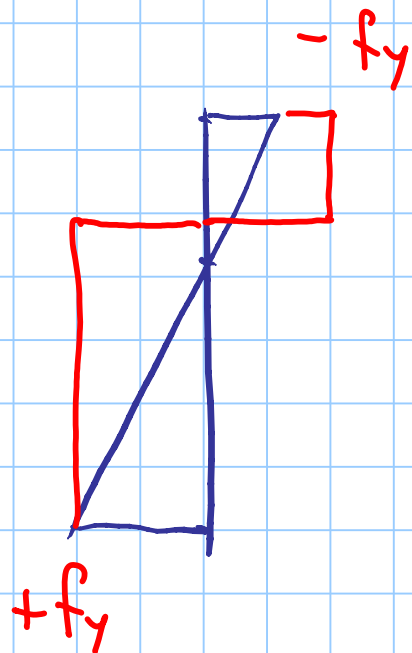
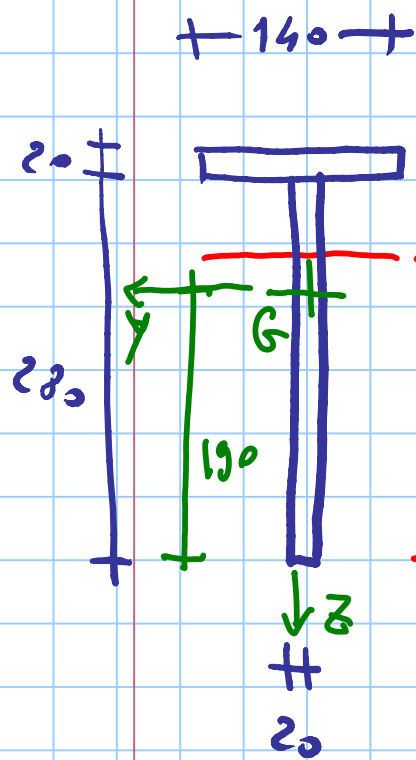
$$M = W f_y$$

$$N = \int \sigma dA = 0$$

$$N = \int_{\substack{\text{tens.} \\ +f_y}} \sigma dA + \int_{\substack{\text{comp.} \\ -f_y}} \sigma dA = f_y \int_{\text{tens.}} dA - f_y \int_{\text{comp.}} dA =$$

$$= f_y A_{\text{tens.}} - f_y A_{\text{comp.}}$$

$$N = 0 \Rightarrow A_{\text{tens.}} = A_{\text{comp.}}$$





$$M_y = \int \sigma z dA = \int_{tens} \sigma z dA + \int_{comp} \sigma z dA =$$

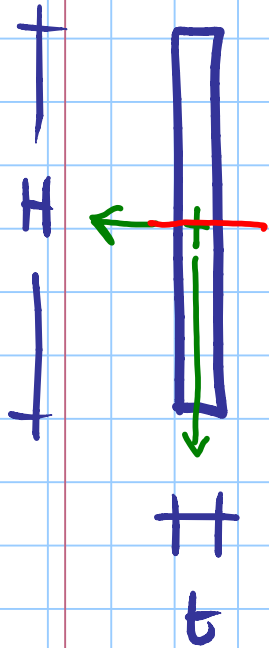
$$= f_y \underbrace{\int_{tens} z dA}_{S_{tens,y}} - f_y \underbrace{\int_{comp} z dA}_{S_{comp,y}} = 2 f_y S_{tens,y}$$

$$S_y = S_{tens,y} + S_{comp,y} = 0 \Rightarrow S_{tens,y} = -S_{comp,y}$$

$$M_{pl,y} = 2 \underbrace{S_{max\,x^2,y}}_{W_{pl}} \cdot f_y$$

in comp. elastic

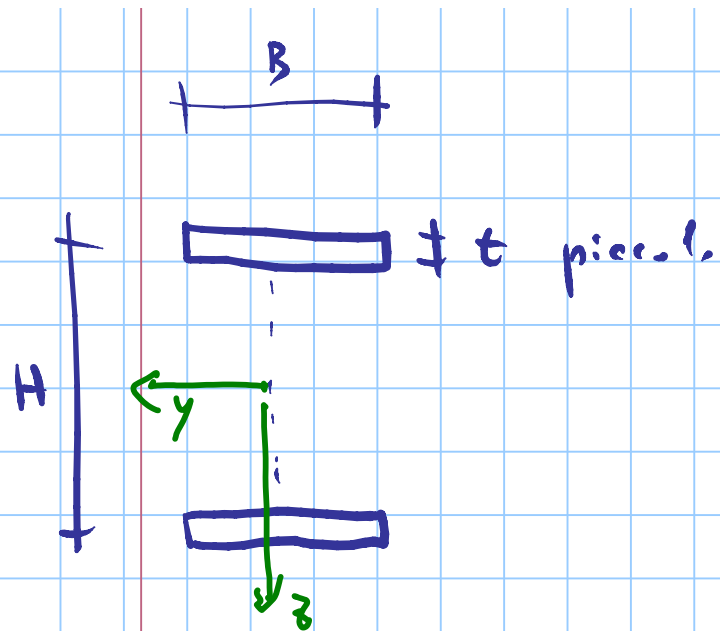
$$M = W_{el} \cdot \sigma$$



$$W_d = \frac{I}{z_{\text{max}}} = \frac{t H^3 / 12}{H/2} = \frac{t H^2}{6}$$

$$W_{pe} = 2 S_{y/2} = 2 \cdot t H/2 \cdot H/4 = \frac{t H^2}{4}$$

$$\frac{W_{pe}}{W_d} = \frac{t H^2 / 4}{t H^2 / 6} = 1.5 \quad \text{fattore di forma}$$



$$W_d = \frac{B t \cdot \left(\frac{H}{2}\right)^2 \cdot 2}{\frac{H}{2}} = B t H$$

Transvers  
 $B t^3 / 12$

anima

$$W_{pl} = 2 \cdot B t \cdot \frac{H}{2} = B t H$$

$$\frac{W_{pl}}{W_d} = 1$$

per section HE, IPE

$$\frac{W_{pl}}{W_{el}} \approx 1.15$$

ex. IPE 180

$$W_{el} = 146.3 \times 10^3 \text{ mm}^3$$

$$W_{pl} = 166.4 \times 10^3 \text{ mm}^3$$

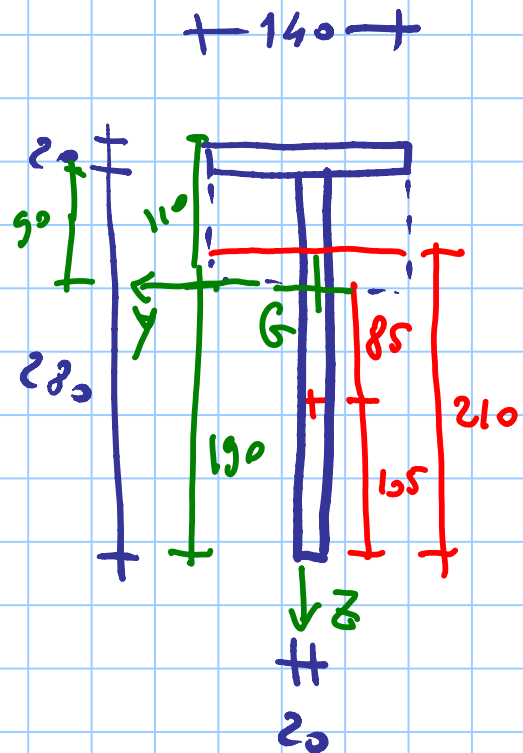
$$\frac{W_{pl}}{W_{el}} = 1.137$$

HEB 180

$$425.7 \times 10^3 \text{ mm}^3$$

$$481.4 \times 10^3 \text{ mm}^3$$

$$1.131$$



$$I = \frac{20 \times 190^3}{3} + \frac{140 \times 110^3}{3} - \frac{120 \times 90^3}{3}$$

$$= 0.7868 \times 10^8 \text{ mm}^4$$

$$W_{el} = \frac{I}{z_{max}} = \frac{0.7868 \times 10^8}{150} = 0.5245 \times 10^6 \text{ mm}^3$$

$$S_{1/2} = 210 \times 20 \times 85 = 0.357 \times 10^6 \text{ mm}^3$$

$$W_{pl} = 2 \times 0.357 \times 10^6 = 0.714 \times 10^6 \text{ mm}^3$$

A hand-drawn diagram of a rectangular cross-section with width B and height H. The coordinate axes are labeled: z (vertical, pointing down) and y (horizontal, pointing left). The origin is at the bottom-left corner.

$$I_z = \frac{B H^3}{3}$$

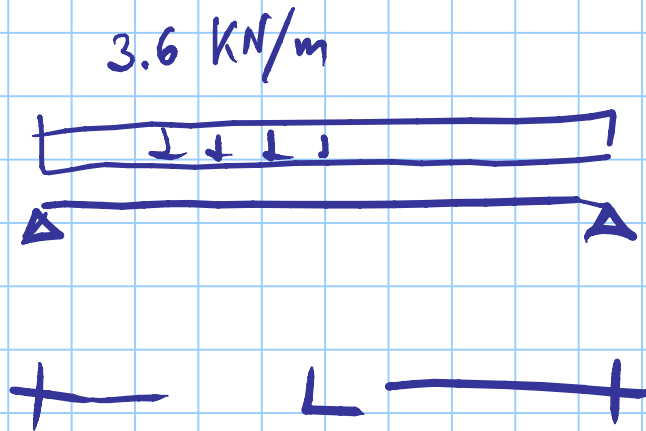
$$M_{pl, Rd} = W_{pl} \frac{f_y}{\gamma_{M0}}$$

VERIFICA

$$M_{Ed} \leq M_{Rd}$$

PROGETTO

$$W_{pl} \geq \frac{M_{Ed}}{f_y / \gamma_{M0}}$$



$$L = 6.00 \text{ m} = 6000 \text{ mm}$$

$$M_{Ed} = \frac{q l^2}{8} = \frac{5.18 \times 6.00^2}{8} = 23.31 \text{ kNm}$$

$$g_k = 1.1 \text{ kN/m}$$

$$q_k = 2.5 \text{ kN/m}$$

$$g_d = 1.1 \times 1.3 = 1.43 \text{ kN/m}$$

$$q_d = 2.5 \times 1.5 = 3.75 \text{ kN/m}$$

$$\underline{\quad \quad \quad}$$

$$5.18 \text{ kN/m}$$



S275

$$W_{pl} \geq \frac{23.31 \times 10^6}{275 / 1.05} = 89.0 \times 10^3 \text{ mm}^3$$

IPE 160       $W_{pl} = 123.9 \times 10^3 \text{ mm}^3$        $I = 869.3 \times 10^4 \text{ mm}^4$

della verifica a deformazione       $I \geq 964.3 \times 10^4 \text{ mm}^4$

non basta IPE 160, serve IPE 180