

MODELLI

CLS

Titolo nota

20/03/2014

1 - LINEARE

TRAZ + COMPR

$$\sigma_c = E_c \varepsilon$$

2 - LINEARE

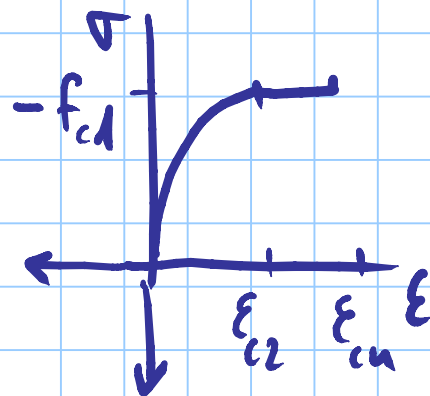
COMPR (no TRAZ)

$$\sigma_c = E_c \varepsilon$$

o.c. per $\varepsilon \leq 0$

3 - NON LINEARE

COMPR (no TRAZ)



$$\varepsilon_{c2} = -2 \times 10^{-3} = -0,002$$

$$\varepsilon_{cu} = -3.5 \times 10^{-3} = -0,0035$$

$$\eta = \frac{\varepsilon_c}{\varepsilon_{c2}}$$

$$0 \leq \eta \leq 1$$

$$\sigma_c = -\eta(2-\eta)f_{cd}$$

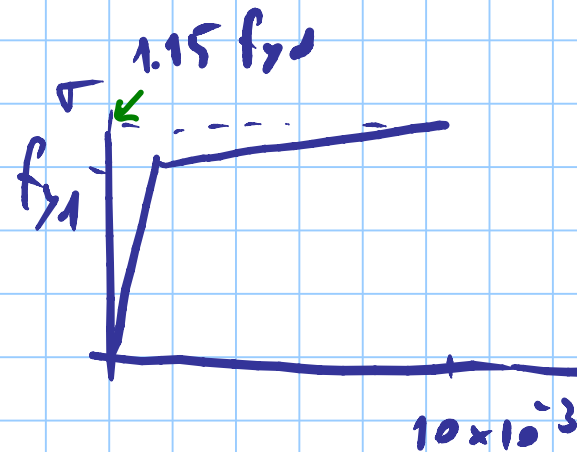
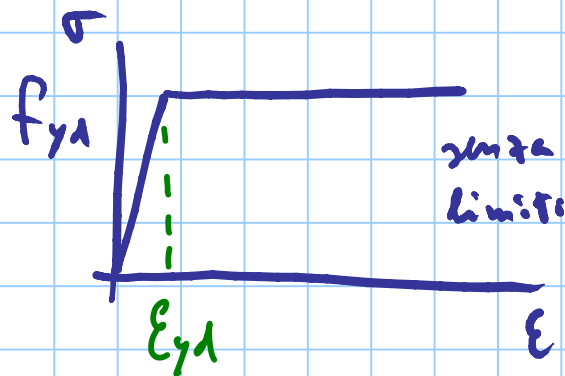
MODELLI

ACCIAIO

1- LINEARE

$$\sigma_c = E_s \varepsilon$$

2- NON LINEARE



B450 C

$$f_{yd} = 391,3 \text{ MPa}$$

$$E_s = 200000 \text{ MPa}$$

$$\varepsilon_{yd} = 1.96 \times 10^{-3}$$

IPOTESI

1 - PERFETTA ADERENZA ACC - CLS

$$\epsilon_c = \epsilon_s \quad \text{in punti inf./n. vicini}$$

2 - MANTENIMENTO DELLA SEZIONE PIANA

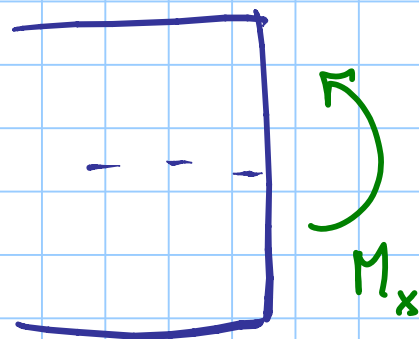
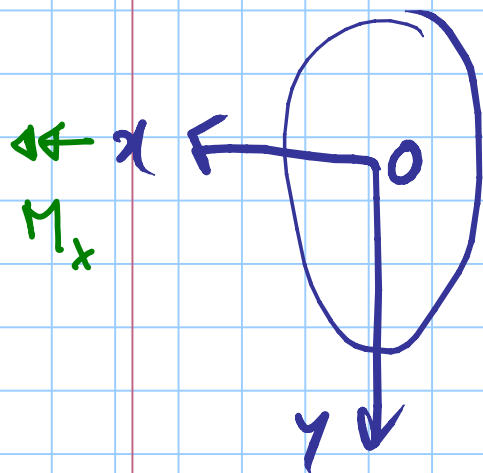
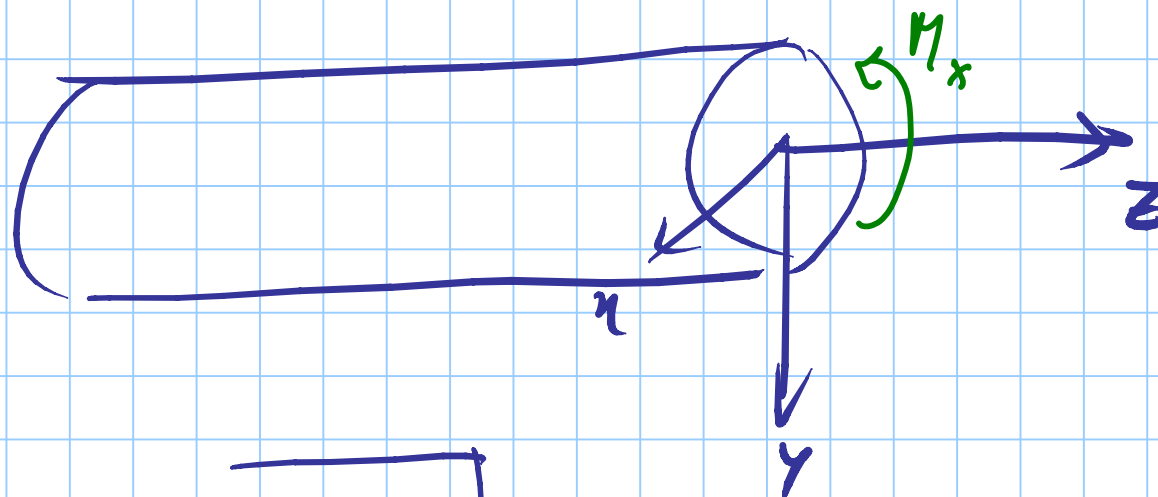
diagramma di ϵ lineare nella sezione

SIMBOLICA

PER

2^a PARTE

CORSO



SEMPRE VALIDE

$$N = \int \sigma dA$$

$$M_x = \int \sigma y dA$$

$$M_y = - \int \sigma x dA$$

Hp. sezione piano

$$\varepsilon = \varepsilon_0 + \left(\frac{1}{2}\right)_x x + \left(\frac{1}{2}\right)_y y$$

$$\left(\frac{1}{2}\right)_x = \frac{\partial \varepsilon}{\partial x}$$

$$\left(\frac{1}{2}\right)_y = \frac{\partial \varepsilon}{\partial y}$$

elasticità
lineare

$$\sigma = E \varepsilon$$

per elasticità lineare

$$\sigma = \frac{N}{A} + \frac{M_x}{I_x} y - \frac{M_y}{I_y} x$$

1° MODELLO DI COMPORT. CLS

$$\left. \begin{aligned} \sigma_c &= E_c \varepsilon_c \\ \sigma_s &= E_s \varepsilon_s \end{aligned} \right\} \text{elasticità lineare}$$

$$\varepsilon_c = \varepsilon_s$$

perfetta aderenza

$$\sigma_s = \underbrace{\frac{E_s}{E_c}}_n \sigma_c$$

$$n = \frac{E_s}{E_c} = \text{coefficiente di OMOGENEIZZAZIONE}$$

$$\sigma_s = n \sigma_c$$

$$N = \int \sigma dA$$

$$\sigma_c dA_c$$

$$\sigma_s dA_s = n \sigma_c dA_s = \sigma_c (n dA_s)$$

faccio finta che l'acciaio sia calcestruzzo

me devo usare un'area n volte maggior

APPLICO LE FORMULE

DELL'ELASTICITA' LINEARE

ALLA SEZIONE OMogeneizzata

$$A_{e,ia} = A_e + n A_s$$

n all'applicazione del carico

$$n = \frac{E_s}{E_c} = 6,35 \quad \text{per } C25/30$$

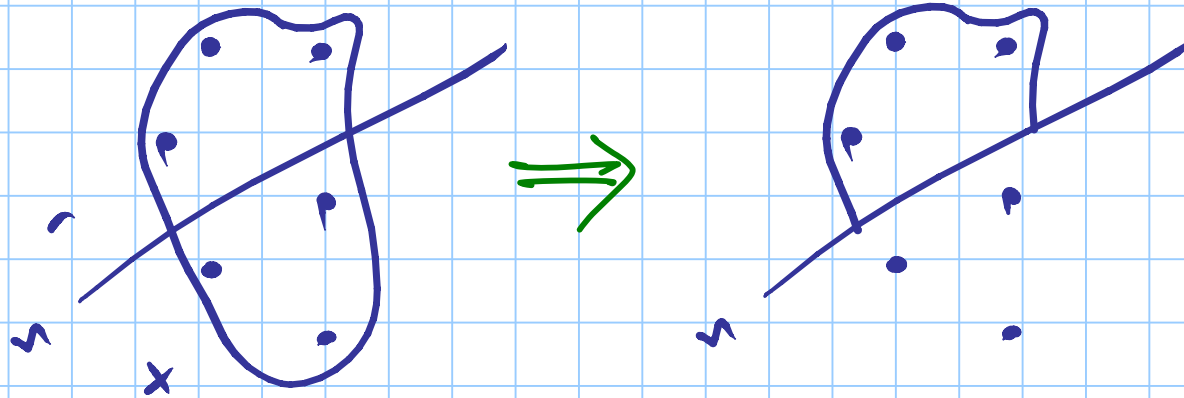
a tempo infinito ϵ crescono

n aumenta $n = 15$

2° modello di comportamento CLS

- CLS LINEARE (no Trazione)

- ACCIAIO LINEARE



SEZIONE
REAGENTE

SI APPLICANO LE FORMULE

DELL' ELASTICITA' LINEARE

ALLA SEZIONE REAGENTE OMOGENEIZZATA

1° problema: qual è la sezione reg. omog. ?

3° m. dello di comportamento

NON possiamo applicare le formule
dell'elasticità lineare

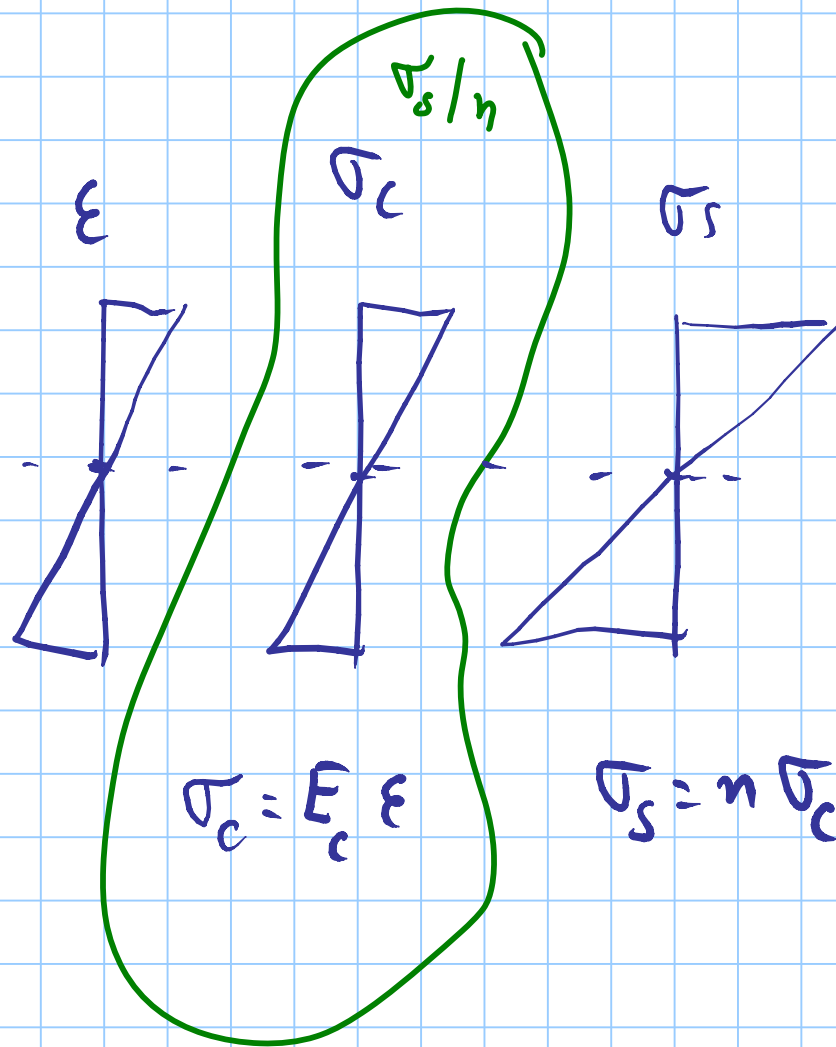
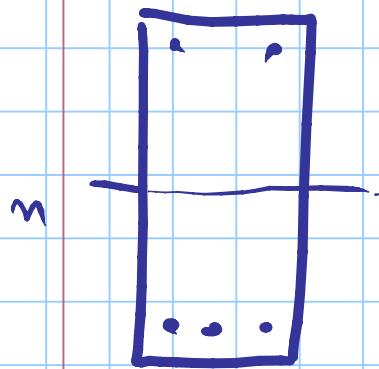
restano $N = \int \sigma dA$

$$M_x = \int \dots$$

$$M_y = \int \dots$$

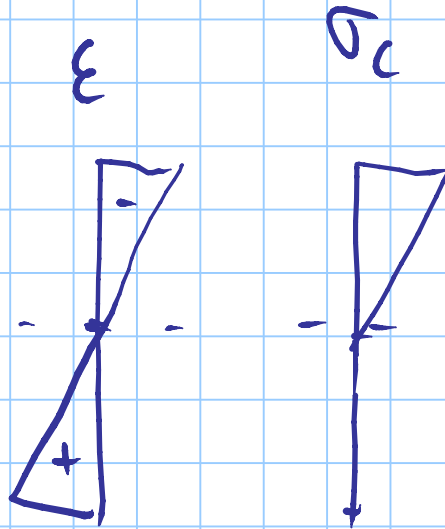
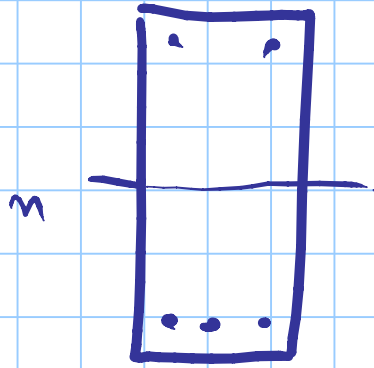
condizioni
di equilibrio

1^o m. dell



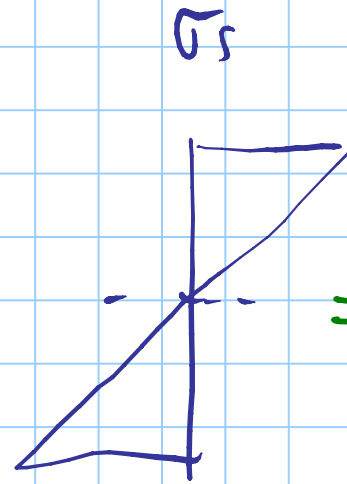
usiamo solo un diagramma

2^o m. all

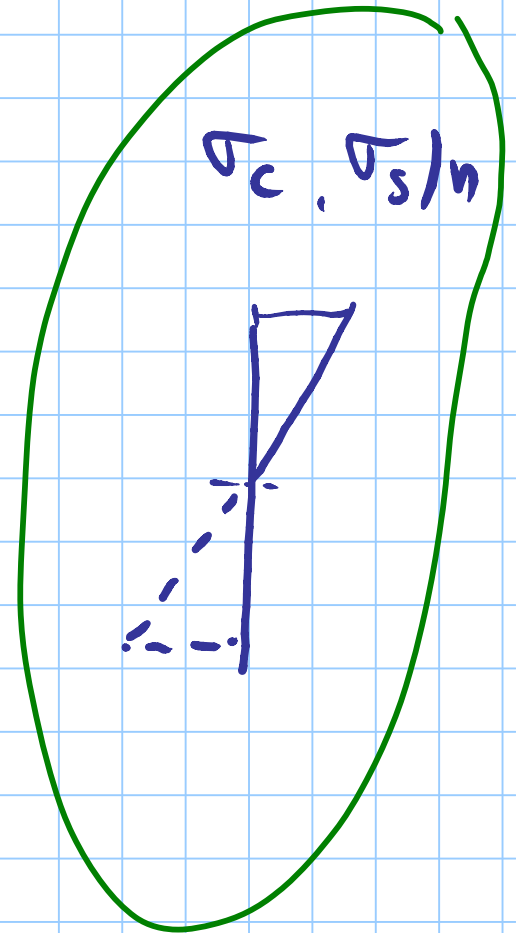


$$\sigma_c = E_c \epsilon$$

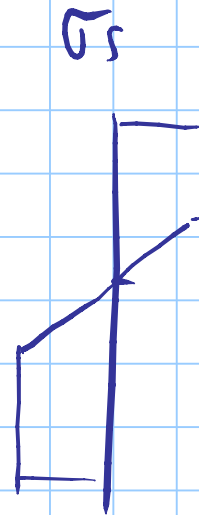
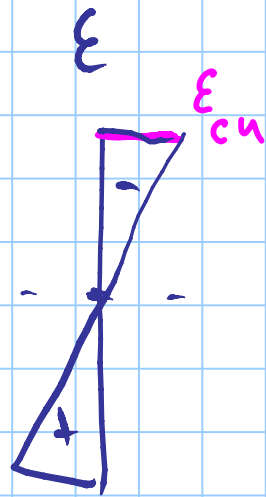
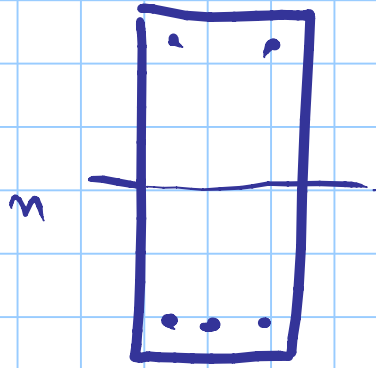
$$\text{per } \epsilon < 0$$



$$\sigma_s = n \sigma_c$$

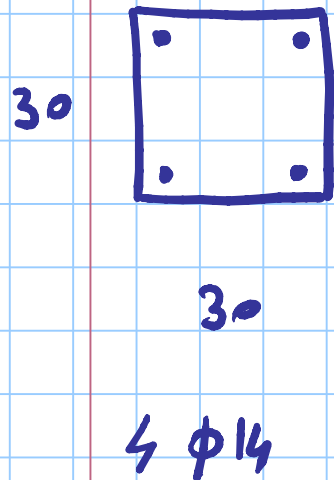


3° m. all



TRAZIONE

1° M.O. COMP.



$$1\phi 14 = 1.54 \text{ cm}^2$$

$$N = 50 \text{ KN (traction)}$$

$$\sigma_c = \frac{N}{A_{ci}} ; \sigma_s = n \sigma_c$$

$$A_{ci} = 30^2 + 4(1.54)n = 939.1 \text{ cm}^2$$

$$n = \frac{E_s}{E_c} = 6.35$$

$$\sigma_c = \frac{N}{A_{ci}} = \frac{50 \text{ kN}}{939,1 \text{ cm}^2} \Leftrightarrow \frac{50 \cdot 10^3}{939,1 \cdot 10^2} = 0,53 \frac{\text{N}}{\text{mm}^2}$$

$$\sigma_s = 6,35 \times 0,53 = 3,38 \frac{\text{N}}{\text{mm}^2}$$

$$f_{ctk} = 1,80 \frac{\text{N}}{\text{mm}^2} > \sigma_c \quad \text{NON FESSURATO}$$

$$\left\{ \begin{aligned} f_{ctk} &= \frac{N_r}{A_{ic}} \rightarrow N = f_{ctk} A_{ic} = 1.80 \times 939.1 / 10 = \\ &= 169 \text{ kN} \end{aligned} \right.$$

$$\left\{ \begin{aligned} \sigma_s &= 6.35 \times \underbrace{1.80}_{f_{ctk}} = 11.43 \frac{\text{N}}{\text{mm}^2} \end{aligned} \right.$$

per lo stato N , subito dopo la formazione

$$N = 169 \text{ KN} \quad \text{Calcestruzzo}$$

NO REAGENTE

$$\sigma_s = \frac{N}{A_s} = \frac{169 \times 10^3}{9 \times 1,34 \times 10^2} = 274,4 \frac{\text{N}}{\text{mm}^2}$$

2° m.d. di comp.

3° m.d. di comput.

$$N_{Rd} = \int \sigma dA = f_{yd} A_s = \frac{391.3 \times 6.16}{10} = 241 \text{ kN}$$

sol
acciaio