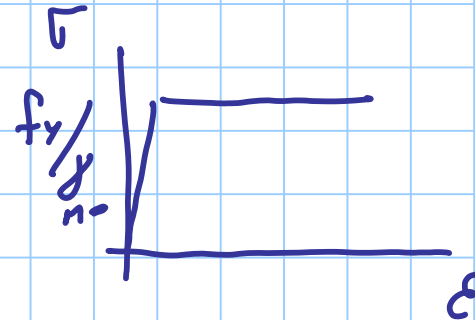


→ $N > 0$ TRAZIONE

$$\sigma = \frac{N}{A}$$

$$\sigma \leq \frac{f_y}{\gamma_{m0}}$$



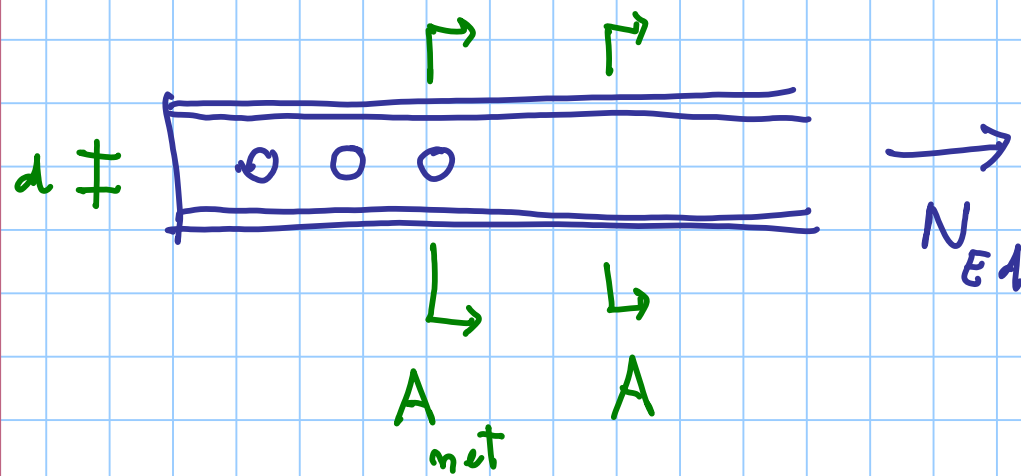
$$N_{t, Rd} = A \frac{f_y}{\gamma_{m0}}$$

VERIFICA

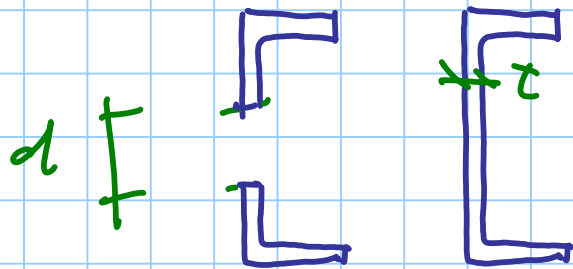
$$N_{Ed} \leq N_{Rd}$$

PROGETTO

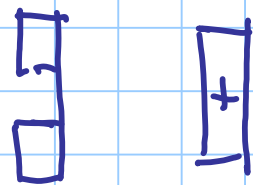
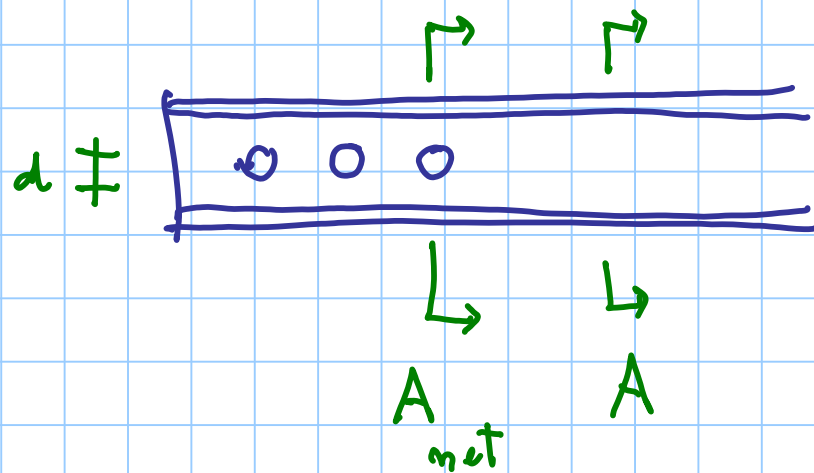
$$A \geq \frac{N_{Ed} \gamma_{m0}}{f_y}$$



d : diameter for
 t = opening



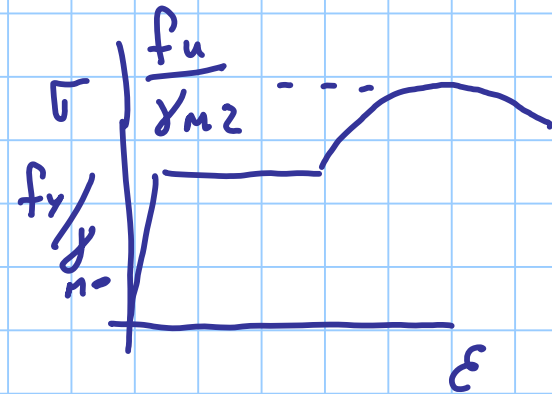
$$A_{net} = A - d t$$



$$\sigma_2 = \frac{N}{A_{net}} \quad \sigma_1 = \frac{N}{A}$$

$$\sigma_2 > \sigma_1$$

N



$$N = A_{net} \frac{f_y}{\gamma_{m2}} \quad \text{movement in comp. and for}$$

$$N_{u,Rd} = 0.9 A_{net} \frac{f_u}{\gamma_{m2}}$$

time cost and for. in $\sigma \neq \text{cost}$

Nelle realtà:

$\sigma \neq \text{cost}$



$$\int \sigma dA < A_{\text{net}} \frac{f_u}{\gamma_{M2}}$$

$$\approx 0.9 A_{\text{net}} \frac{f_u}{\gamma_{M2}}$$

$$N_{Rd} = \min \left(N_{t,Rd} , N_{u,Rd} \right)$$

/

dimension
section standard

|

rotture
section pref.

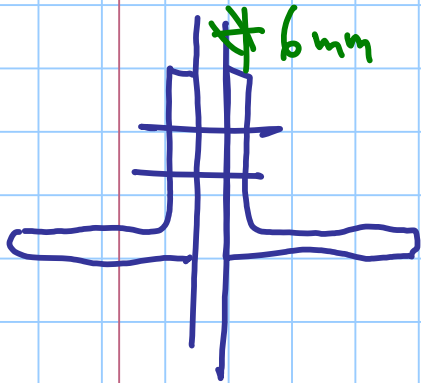
se $N_{u,Rd} < N_{t,Rd}$ comportamento FRAGILE

se $N_{t,Rd} < N_{u,Rd}$ comportamento DUTTILE

average project. $N_{Ed} = 300 \text{ KN} < N_{t,Rd}$
OK

2 L 60x60x6

$f_{y,235}$ $d_o = 13 \text{ mm}$



$$N_{t,Rd} = \frac{A \cdot \frac{f_y}{\gamma_{m,235}}}{13.82 \times 10^2 \text{ mm}^2} = 309.3 \text{ KN}$$

$$A_{net} = A - 2dt = 13.82 \times 10^2 - 2 \times 13 \times 6 = 12.26 \times 10^2 \text{ mm}^2$$

$$N_{u,Rd} = 0.9 A_{net} \frac{f_y}{\gamma_{M2}} = 0.9 \times 12.26 \times 10^2 \times \frac{360}{1.25} \times 10^{-3}$$

$$= 317.8 \text{ kN}$$

l'asta può portare $N_{Ed} = 300 \text{ kN}$

comportamento duttile perché

$$\underset{309.3}{N_{t,Rd}} < \underset{317.8}{N_{u,Rd}}$$

$$N_{t,R1} < N_{u,R1}$$

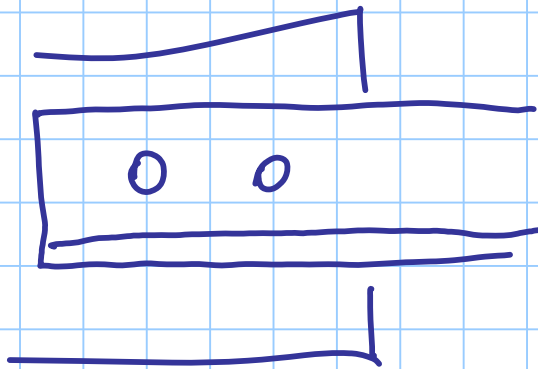
DUCTILE

$$A \frac{f_y}{\gamma_{m0}} < 0.9 A_{net} \frac{f_u}{\gamma_{m2}}$$

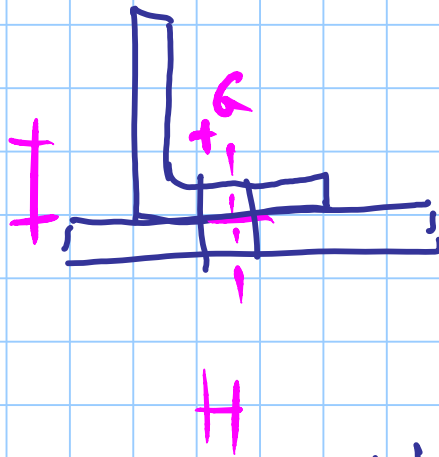
$$\frac{A_{net}}{A} > \frac{f_y / \gamma_{m0}}{0.9 f_u / \gamma_{m2}}$$

$$= \frac{f_y \gamma_{m2}}{0.9 f_u \gamma_{m0}}$$

$$\begin{aligned} S235 &= 0.863 \\ S275 &= 0.846 \\ S355 &= 0.921 \end{aligned}$$

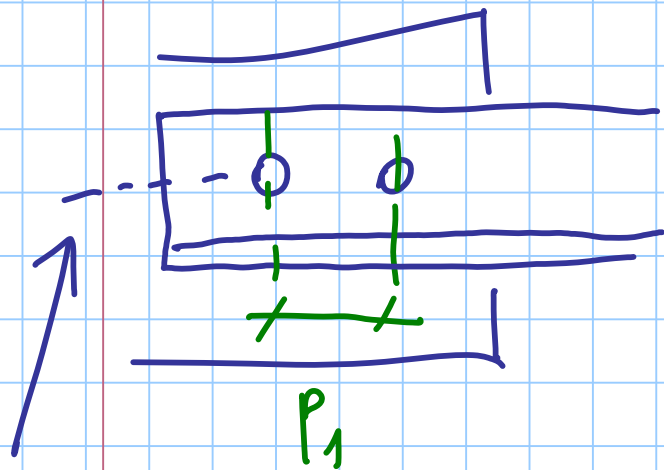


UNA SOLA L



la sezione di collegamento
è soggetta a M, N

$$N_{u, Rd} = \beta A_{net} \frac{f_y}{\gamma_{m2}}$$



se vi sono 2 bulloni

ASSE DI TRUSCHINO

$$\beta = \beta_2$$

$$p_1 \leq 2.5 d_o$$

$$\beta_2 = 0.4$$

$$p_1 \geq 5 d_o$$

$$\beta_2 = 0.7$$

per 3 bulloni

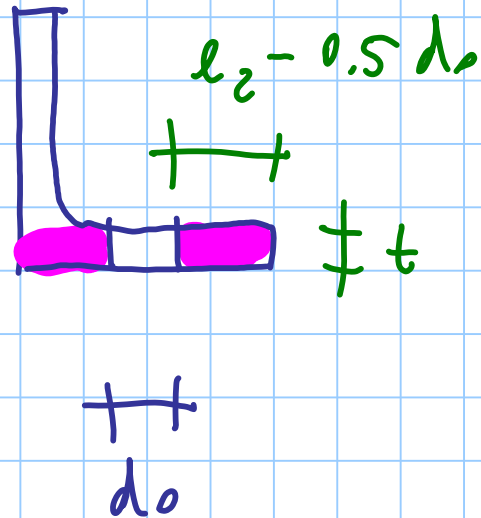
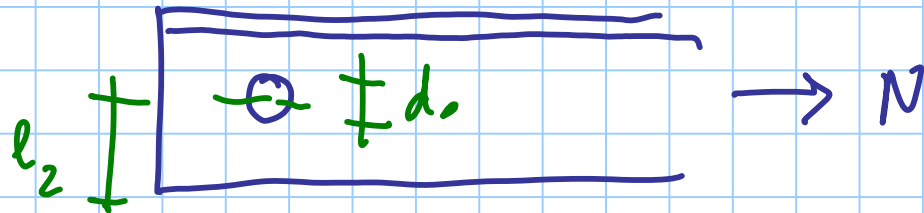
$$d_o > 3$$

$$\beta = \beta_3$$

$$\beta_3 = 0.5$$

$$\beta_3 = 0.7$$

per un solo bullone



$$N_{u,R1} = 2(l_2 - 0.5 d_o) t \frac{f_u}{\gamma_{M2}}$$

FLESSIONE SEMPLICE

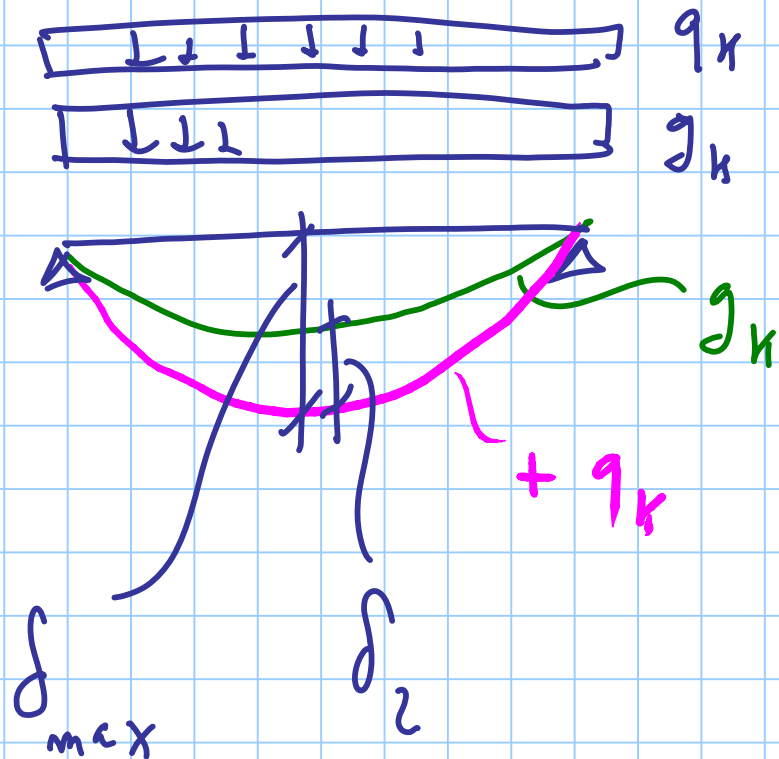
VERIFICA SLU

comportamenti
condizioni di instabilità locale \rightarrow classi

- 1 } piena
- 2 } plasticizz.
- 3 | al limite f_y
- 4

VERIFICA SLE

deformazioni



si accettano. invece
doppio per valori

copertura

valori praticabili

con travi in legno

δ_{max}

δ_2

$$\frac{1}{200} L$$

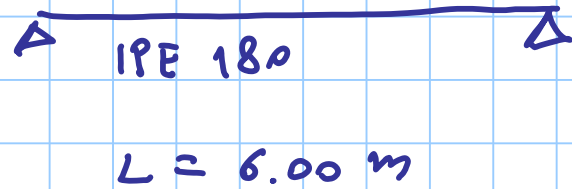
$$\frac{1}{250} L$$

$$\frac{1}{250} L$$

$$\frac{1}{300} L$$

$$\frac{1}{250} L$$

$$\frac{1}{350} L$$



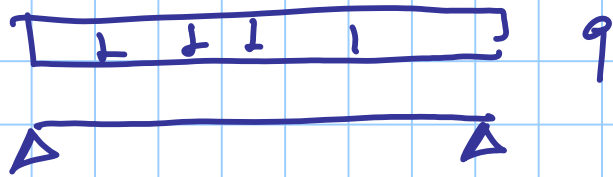
$$g_k = 0.5 \text{ kN/m}$$

$$q_k = 1.6 \text{ kN/m}$$

limit I: $\delta_{max} = \frac{1}{200} L = 30 \text{ mm} [q = 2.1 \text{ kN/m}] E = 210000 \text{ MPa}$

$$\delta_2 = \frac{1}{250} L = 24 \text{ mm} [q = 1.6 \text{ kN/m}]$$

IPE 180 $I_y = 1317 \times 10^4 \text{ mm}^4$



$$\delta = \frac{5}{384} \frac{q L^4}{EI}$$

$$(6 \times 10^3)^4 = 6^4 \times 10^{12}$$

$$\delta_{max} = \frac{5}{384} \frac{2.1 \times 6.00^4 \times 10^{12}}{210000 \times 1317 \times 10^4} = 12.8 \text{ mm}$$

$$\delta_2 = 9.76 \text{ mm}$$

$$\frac{5}{384} \frac{q L^4}{EI} \leq \delta_{max}$$

$$I \geq \frac{5}{384} \frac{q L^4}{E \delta_{max}} = \frac{5 \times 2.1 \times 6^4 \times 10^{12}}{384 \times 210000 \times 30} = 562.5 \times 10^4$$

$$\text{con } \delta_l \quad \frac{1.6}{24} \rightarrow$$

$$\text{IPE 160 } I = 869.3 \times 10^4$$

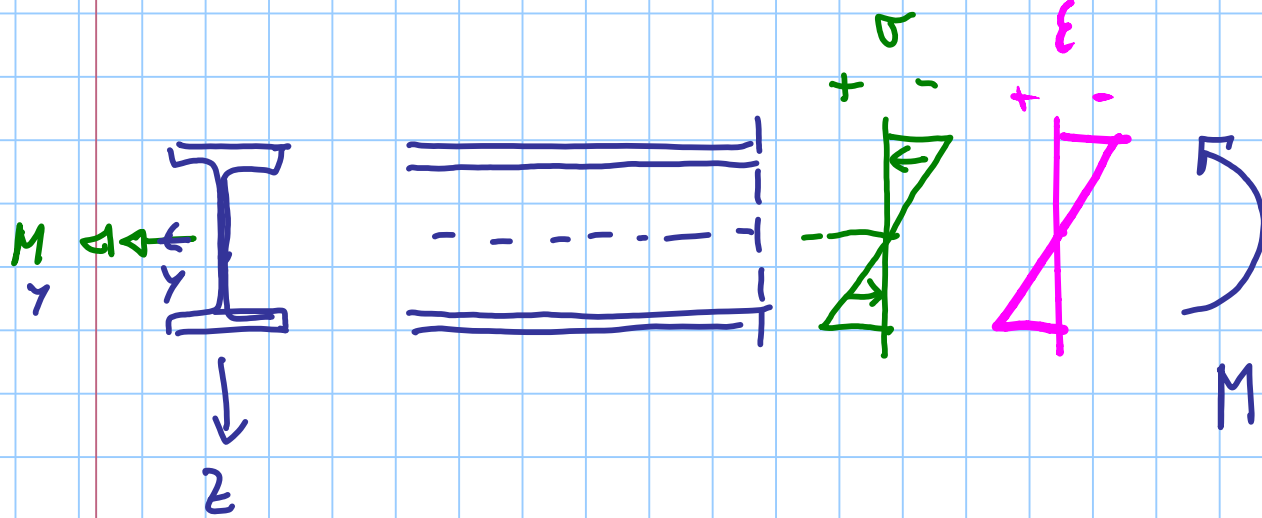
$$\text{IPE 140 } I = 541.2 \times 10^4$$

$$\text{HE 120 A } I = 606.2 \times 10^4$$

OK IPE 160

pes. 0.158 kN/m

° HE 120 A 0.199 kN/m



$\sigma = E \epsilon$
 elastic
 linear

$$\epsilon = \epsilon_0 + \frac{d\epsilon}{dy} y + \frac{d\epsilon}{dz} z$$

χ

$\frac{1}{z}$

$$N = \int_A \sigma dA$$

$$M_y = \int_A \sigma z dA$$

$$M_z = - \int_A \sigma y dA$$

$$N = \int E (\epsilon_x + \chi_y y + \chi_z z) dA =$$

$$= E \epsilon_x \underbrace{\int dA}_A + E \chi_y \underbrace{\int y dA}_{\text{zero}} + E \chi_z \underbrace{\int z dA}_{\text{zero}}$$

$$N = E \epsilon_x A$$

$$\epsilon_x = \frac{N}{EA}$$

$$M_y = \int E (\epsilon_t + \chi_y y + \chi_z z) z dA =$$

$$= E \epsilon_t \underbrace{\int z dA}_{z_{th}} + E \chi_y \underbrace{\int y z dA}_{z_{th}} + E \chi_z \underbrace{\int z^2 dA}_{I_y}$$

$$M_y = E \chi_z I_y$$

$$\chi_z = \frac{M_y}{E I_y}$$

$$\chi_y = - \frac{M_z}{E I_z}$$

$$\xi = \xi_0 + \chi_y y + \chi_z z =$$

$$= \frac{N}{EA} - \frac{M_z}{EI_z} y + \frac{M_y}{EI_y} z$$

$$\sigma = E \xi = \frac{N}{A} + \frac{M_y}{I_y} z - \frac{M_z}{I_z} y$$

NAVIER

$$\sigma = \frac{M_y}{I_y} z$$

$$\sigma_{max} = \frac{M_y}{I_y} z_{max} = \frac{M_y}{W_y}$$

per sezioni di classe 3

modello lineare

limite $\sigma = \frac{f_y}{\gamma_m}$

$$M_{Rd} = W_{el,y} \frac{f_y}{\gamma_m}$$

classe 1 e 2

comportamento plastico

sezione piana

legame lineare

SI

NO

