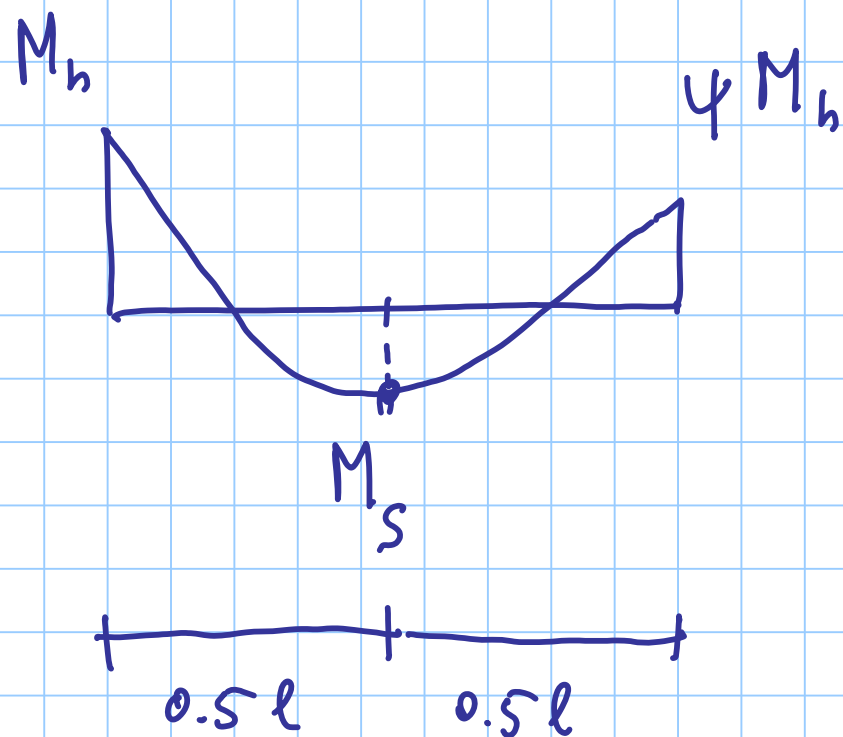


me F. 6 A



M_m

$$M_{eq} = 1.3 M_m \geq 0.75$$



CARICO

— uniforme

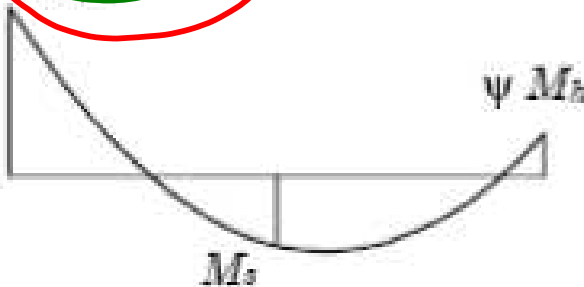
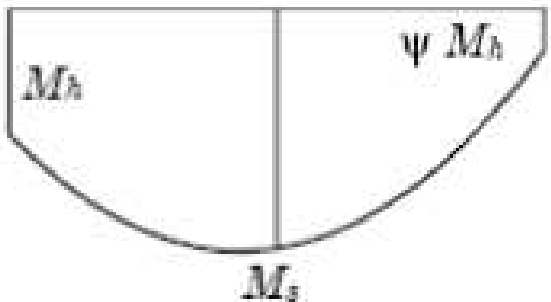
— non uniforme

VALORE MASSIMO

— $|M_h|$

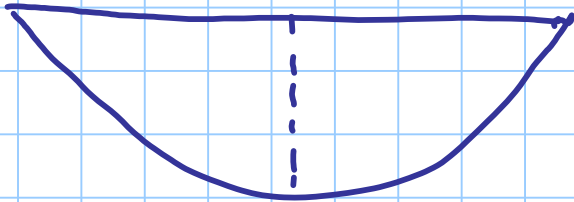
— $|M_s|$

Tab. 1. Valori di α_m nel caso di carico uniforme

Diagramma del momento	Intervallo		α_m
<p>$M_h > M_s$ $\alpha_s = M_s / M_h$</p> 	$0 \leq \alpha_s \leq 1$	$-1 \leq \psi \leq 1$	$0.2 + 0.8 \alpha_s \geq 0.4$
	$-1 \leq \alpha_s < 0$	$0 \leq \psi \leq 1$	$0.1 - 0.8 \alpha_s \geq 0.4$
		$-1 \leq \psi < 0$	$0.1(1 - \psi) - 0.8 \alpha_s \geq 0.4$
<p>$M_s > M_h$ $\alpha_h = M_h / M_s$</p> 	$0 \leq \alpha_h \leq 1$	$-1 \leq \psi \leq 1$	$0.95 + 0.05 \alpha_h$
	$-1 \leq \alpha_h < 0$	$0 \leq \psi \leq 1$	$0.95 + 0.05 \alpha_h$
		$-1 \leq \psi < 0$	$0.95 + 0.05 \alpha_h (1 + 2\psi)$

$$M_h = 0$$

$$\psi = 1$$



$$M_s = \frac{ql^2}{8}$$

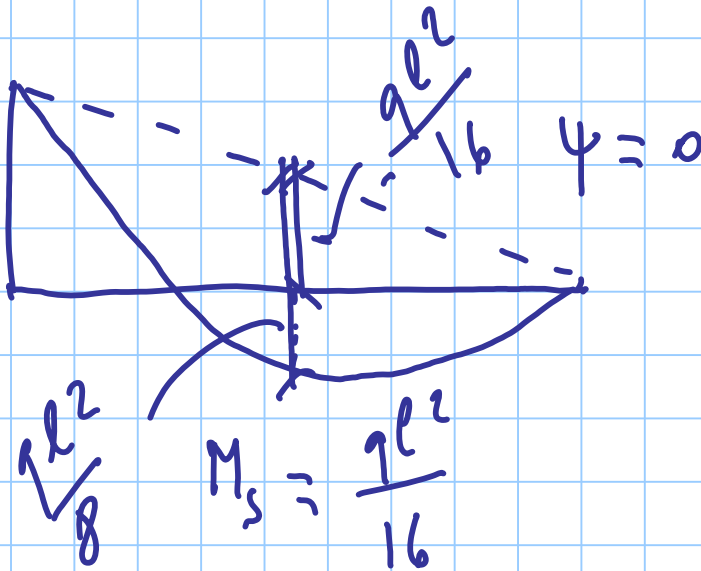
$$|M_s| > |M_h|$$

$$\alpha_h = 0$$

$$\alpha_m = 0.95 + 0.05 \alpha_h =$$

$$= 0.95$$

$$M_h = \frac{ql^2}{8}$$



$$\psi = 0$$

$$\frac{ql^2}{16}$$

$$\frac{ql^2}{8}$$

$$M_s = \frac{ql^2}{16}$$

$$|M_h| > |M_s|$$

$$\alpha_s = -0.5$$

$$\alpha_m = 0.1 - 0.8 \alpha_s = 0.5$$

$$M_h =$$

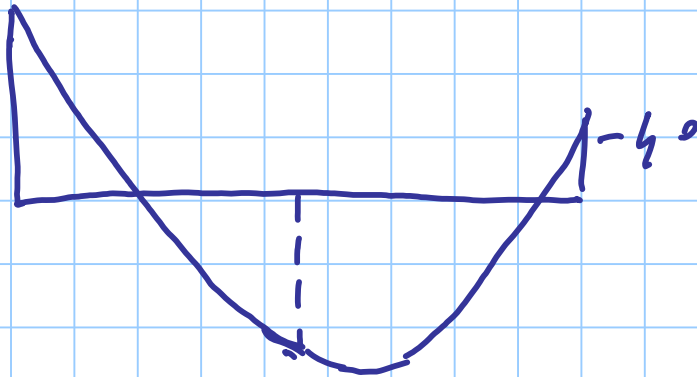
$$-100$$

$$\psi = 0.4$$

$$|M_h| > |M_s|$$

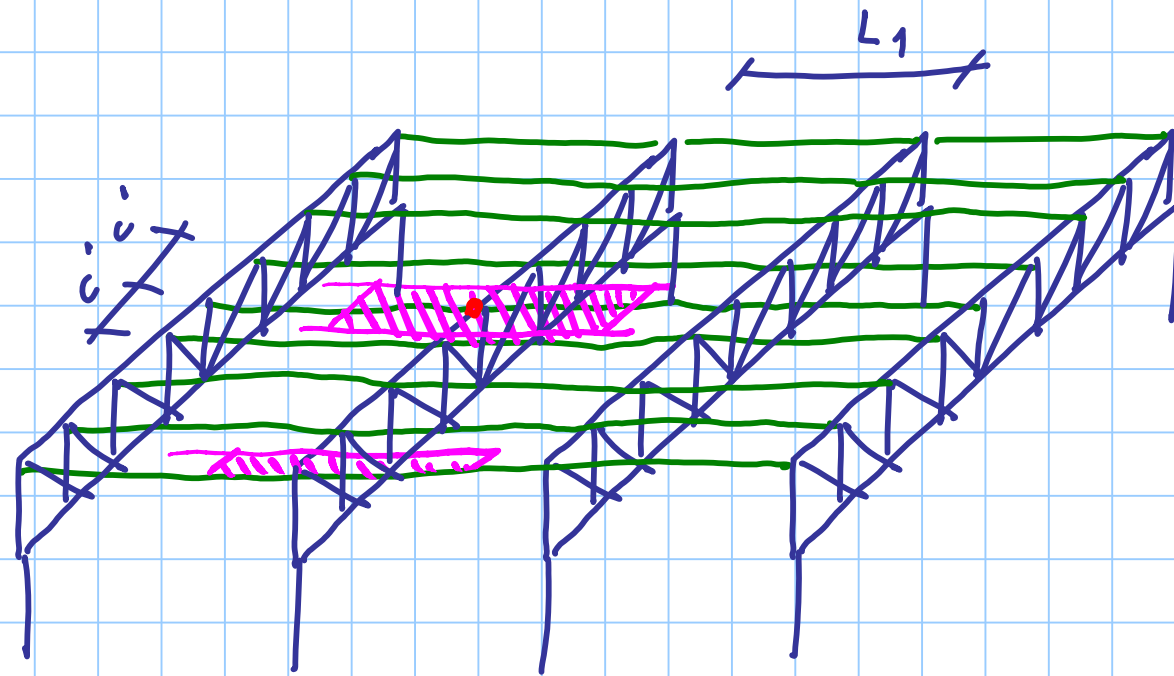
$$\alpha_s = -0.7$$

$$\alpha_n = 0.1 - 0.8 \alpha_s = 0.66$$



$$M_s = 70$$

PROGETTO



CARICO SUL NODO

$$i = 2.20 \text{ m} \quad L_1 = 5.80 \text{ m}$$

G_k Q_k

copertura $i \times L_1 = 12.76 \text{ m}^2$

- neve + mant.
h (0.74 kN/m^2)

9.44 kN

- vento
k (-0.64 kN/m^2)

- 8.17 kN

- pes. pannello (0.1 kN/m^2)

1.28 kN

trave secondaria $L_1 = 5.8 \text{ m}$

- p.p. (0.26 kN/m)

1.51 kN

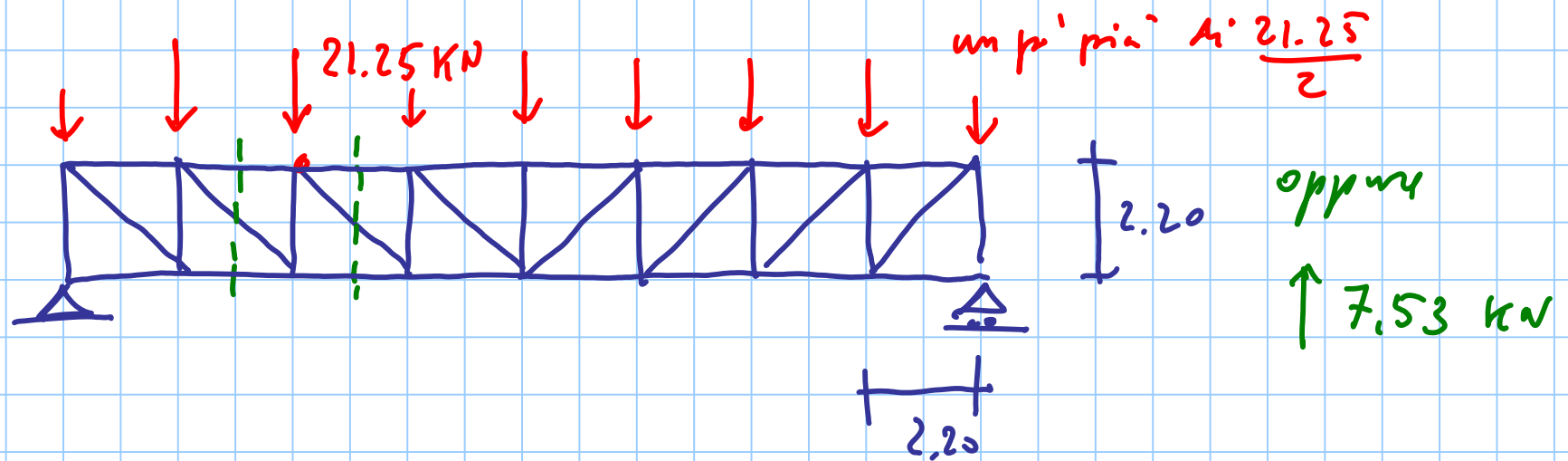
VALORI DI CALCOLO del carico sul nodo

verso il basso

$$(1.28 + 1.51) \times 1.3 + 9.44 \times 1.5 =$$
$$= 18.73 \text{ kN} + 2.52 = 21.25 \text{ kN}$$

verso l'alto

$$(1.28 + 1.51) \times 1.3 - 8.17 \times 1.5 =$$
$$= -9.47 \text{ kN} + 1.94 = -7.53 \text{ kN}$$



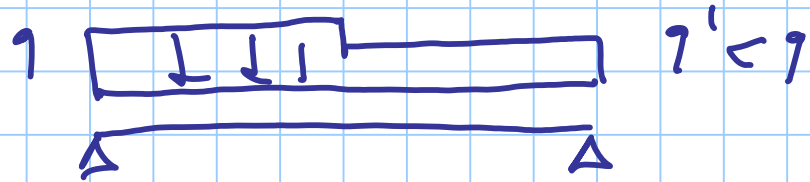
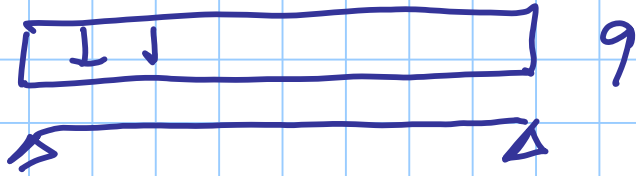
corrente sup	2.20 m
" inf	2.20 m
montante	2.20 m
diagonale	$2.20 \sqrt{2} = 3.10$ m

Total 9.70 m

$\times 0.2$ kN/m

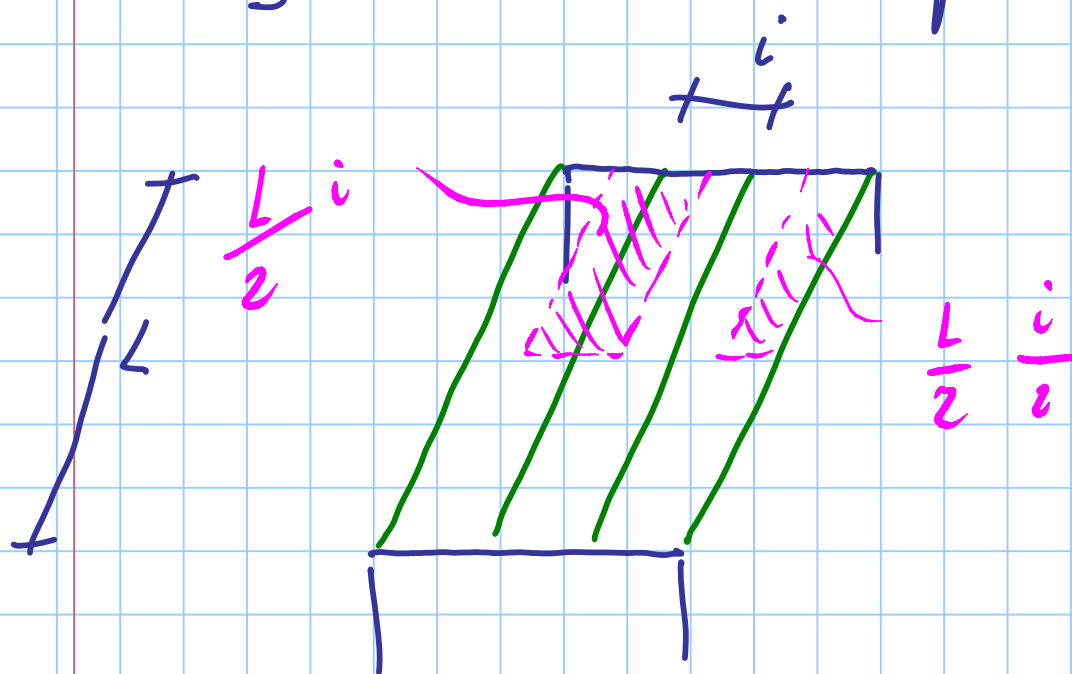
$$g_k = 1.94 \text{ kN}$$

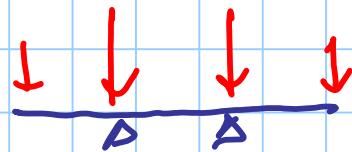
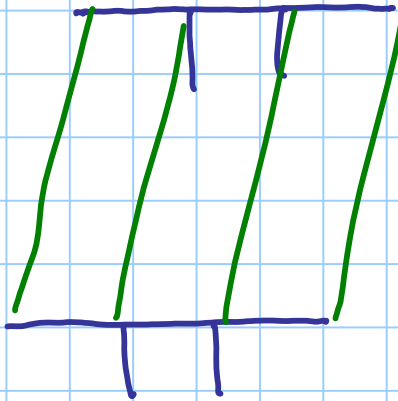
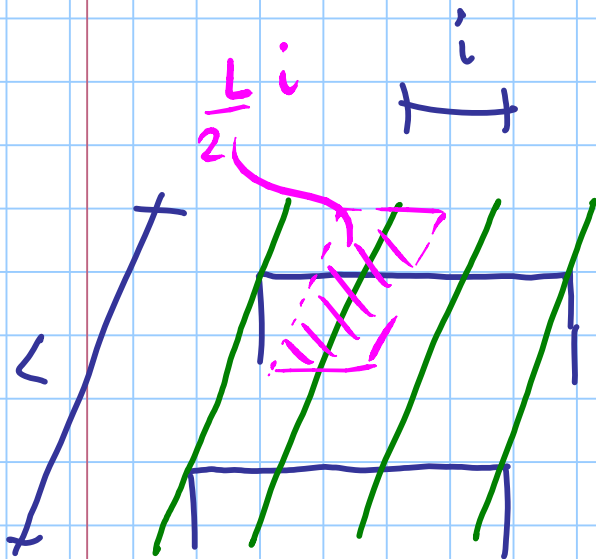
$$g_d = 1.94 \times 1.3 = 2.52 \text{ kN}$$



SCALA

coperture - 2 travi principali





RAMPE

