

solai  
interni  
abitazioni

carichi perm. strutturali  
+ non strutt. comp. def.

$$g_1 \sim 6.1 \text{ kN/m}^2$$

carichi non strutt. (incisioni)  
non comp. int. def. (tramezzi)

$$g_1 \sim 1.8 \text{ kN/m}^2$$

carichi variabili

$$g_1 \sim 3.0 \text{ kN/m}^2$$

balconi  
terrazze

carichi perm.

$$g_1 \sim 5.8 \text{ kN/m}^2$$

carichi variabili

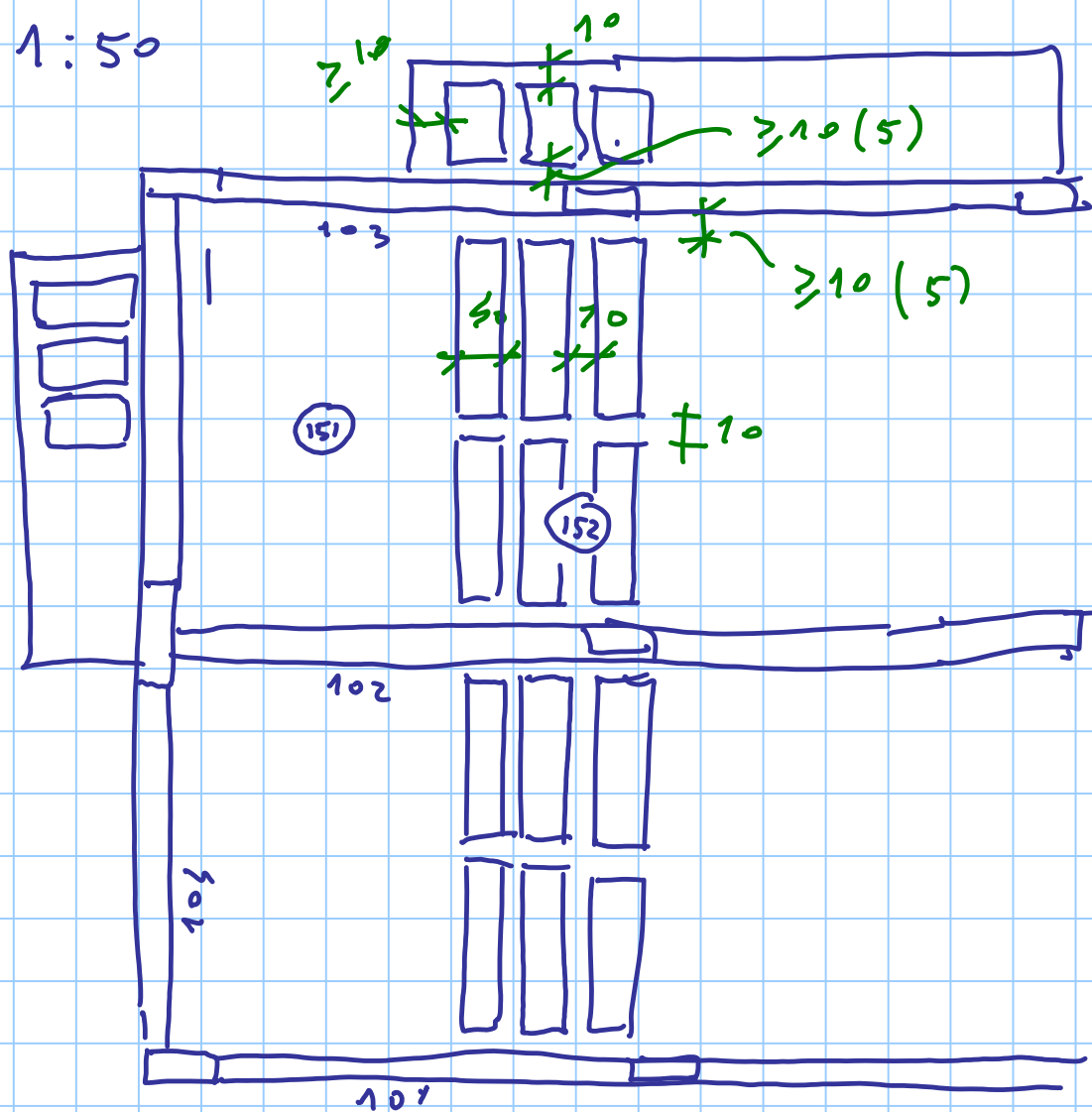
$$g_1 \sim 6.0 \text{ kN/m}^2$$

tampone

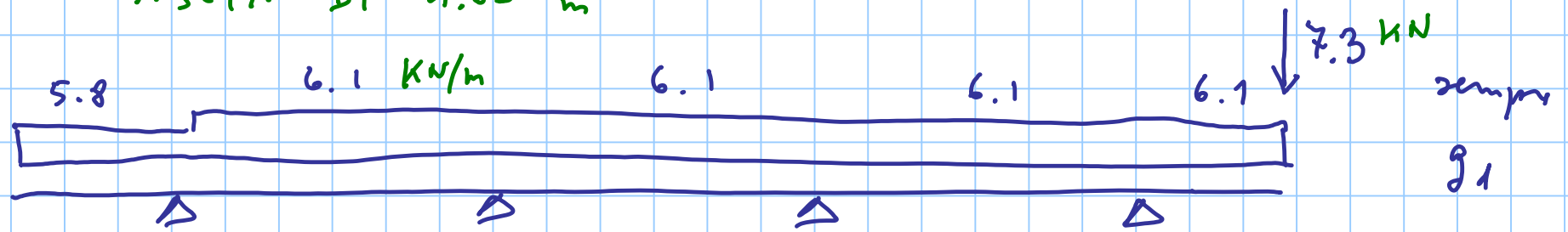
$$g_1 = 7.3 \text{ kN/m}$$

portone  
non carico

1:50



FASCEIA DI 1.00 m



	estern		intern.				intern.	
1)	11.8		6.1		10.9		6.1	10.9 ↓ 7.3
2)	5.8		10.9		6.1		10.9	6.1 ↓ 7.3
3)	5.8		10.9		10.9		6.1	10.9 ↓ 7.3
4)	11.8		6.1		10.9		10.9	6.1 ↓ 7.3

4 conditioni di carico per Trave continua

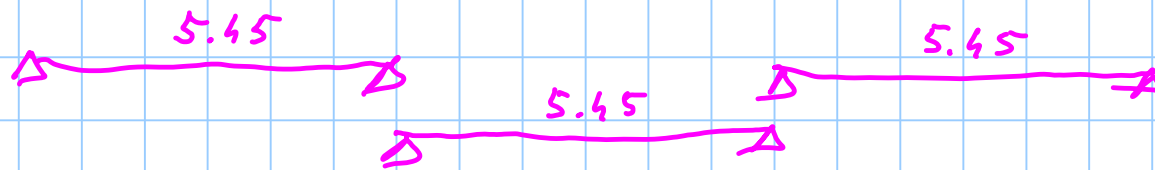
diagrammi dx f  
con Traveca

5)

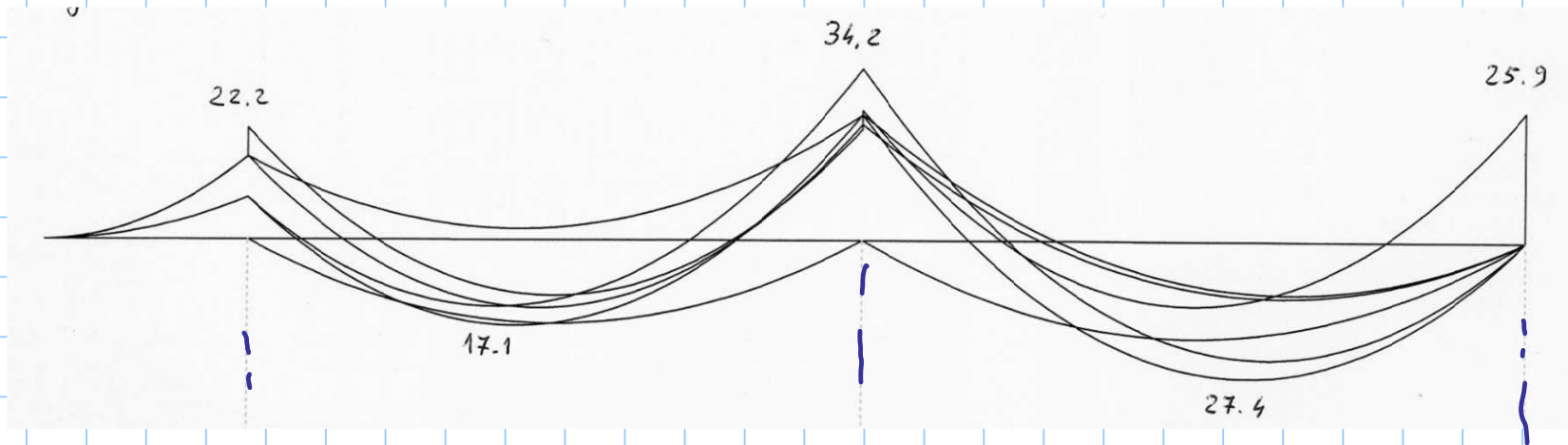


SCHEM. LIMITE

c)

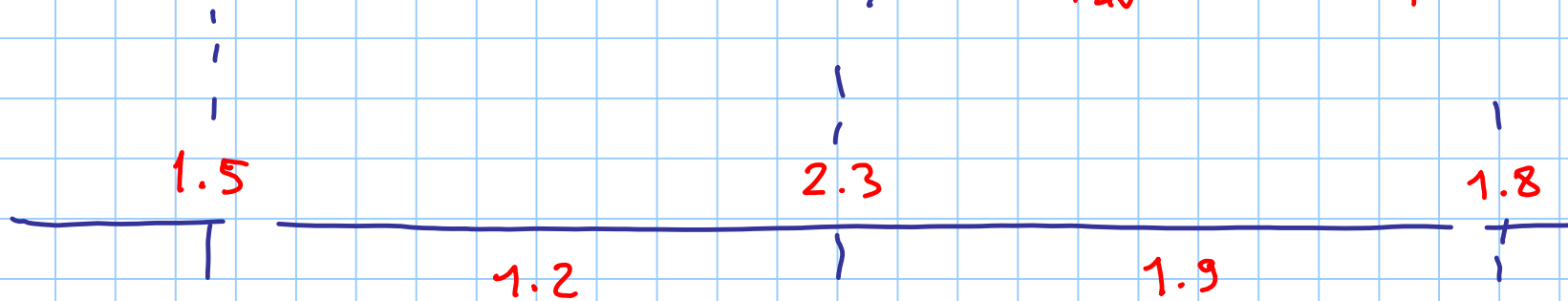


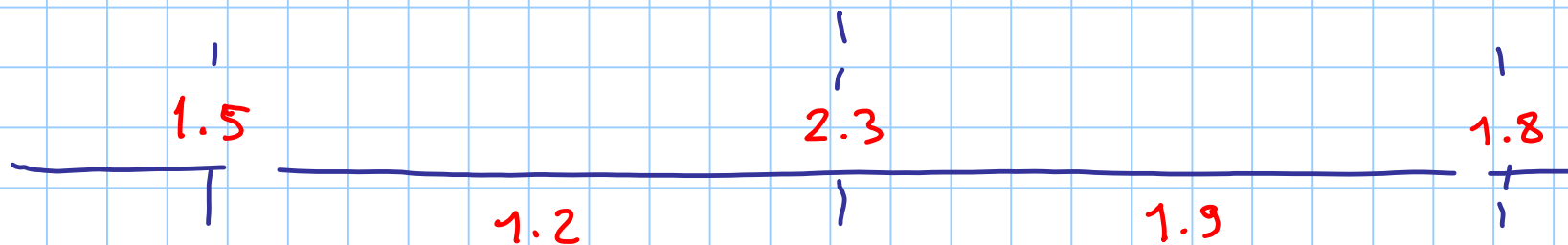
diagr. dxf  
con Mthm cad



scala lunghezza 1:20 (di solito)

calcolo armatura  $A_s = \frac{M}{0.9 d f_{yd}} \cdot \frac{1}{\eta_{trv}} \rightarrow$  armatura per trave





$$\phi_{10} \rightarrow 0.79 \text{ m}^2$$

$$\phi_{15} \rightarrow 1.59 \text{ m}^2$$