

COLLEGAMENTO (n. controvento - Trave)

sollecitazione $N_{Ed} = 400 \text{ kN}$

resistenza dell'asta UPE140 $A = 18.4 \times 10^2 \text{ mm}^2$ S275

$$N_{Rd} = 18.4 \times 10^2 \times \frac{275}{1.05} \times 10^{-3} = 481.9 \text{ kN}$$

Per avere sovraamento asta prima delle rotture del collegamento:

PROGETTARE COLLEGAMENTO per 481.9 kN

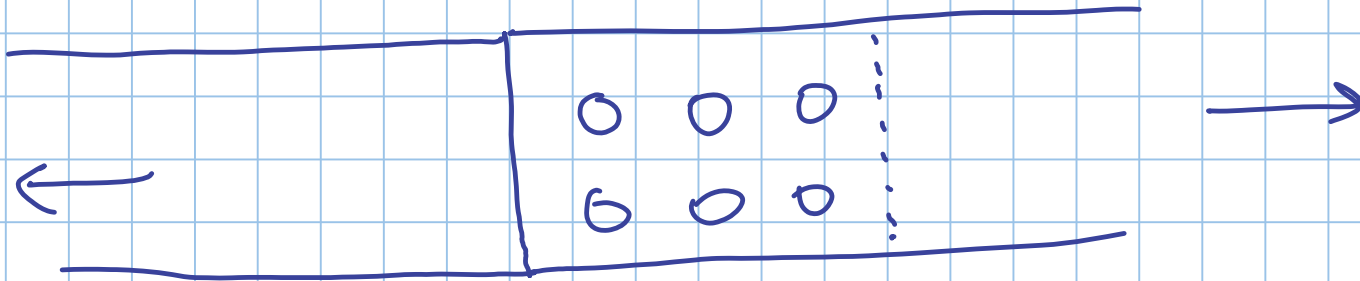
COLLEGAMENTO

a completo ripristino di resistenza

se più resistente dei pezzi che unisce

ovvero

progetto in base alla resistenza dei pezzi



I bulloni lavorano a taglio.

dopo aver superato la resistenza per attrito

Alternativa

Verificare che non si abbia scorrimento

Progettare il collegamento in modo che la resistenza per ATTRITO
sia maggiore dell'azione sollecitante

Bulloni ben serrati

Forze di serraggio

$$F_p = 0.7 A_{res} \frac{f_{ub}}{\gamma_{M7}}$$

($\gamma_{M7}=1$ a serraggio controllato)

Resistenza per vite

$$F_{S,Rd} = \frac{\mu F_p}{\gamma_{M3}}$$

$$\mu = 0.2$$

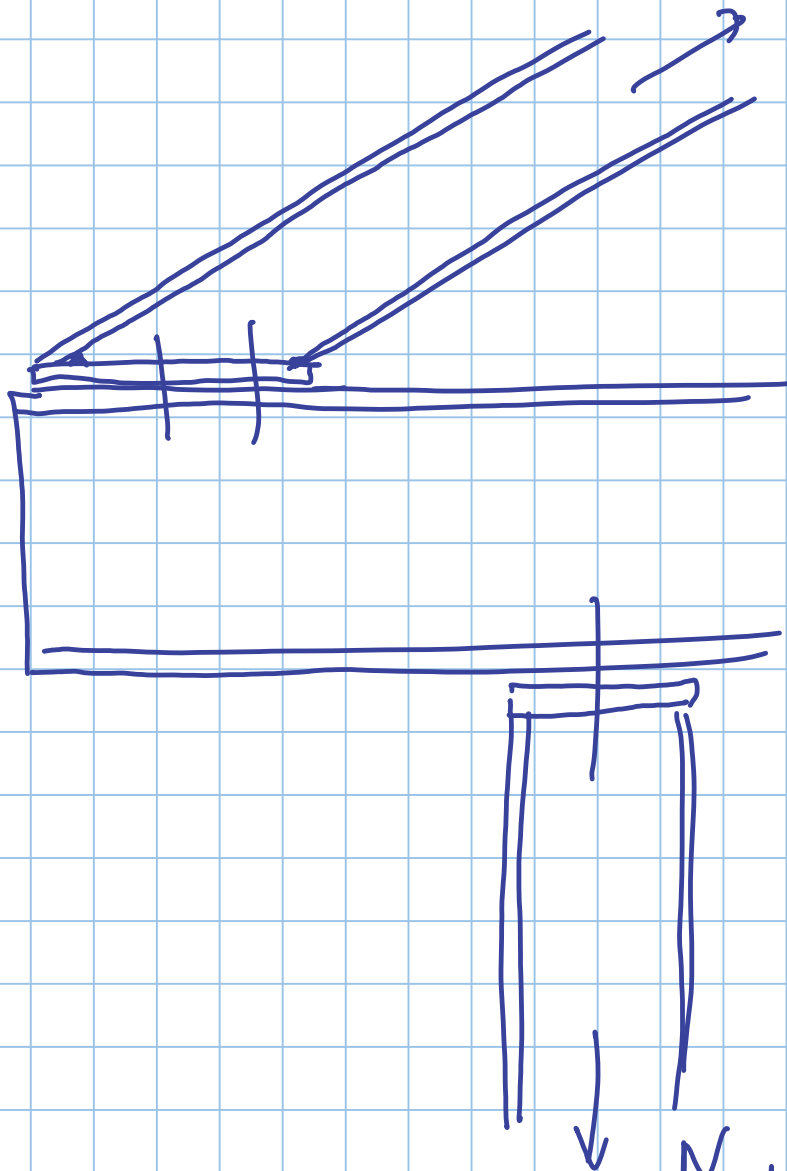
$$0.3$$

$$0.4$$

$$0.5$$

da progettare per SLU

oppure per SLE



bulloni che lavorano
a taglio e a trazione

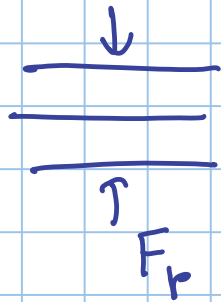
bulloni che lavorano
a trazione

$$N_{Ed} = 400 \text{ kN}$$

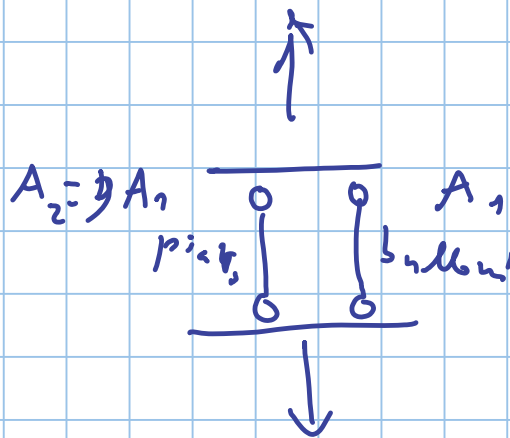
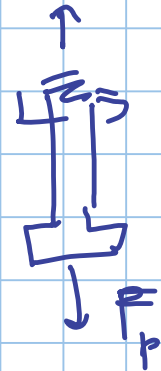
Resistenza di un bullone tes.

$$F_{t,Rd} = 0.9 A_{res} \frac{f_{ub}}{\gamma_{M2}}$$

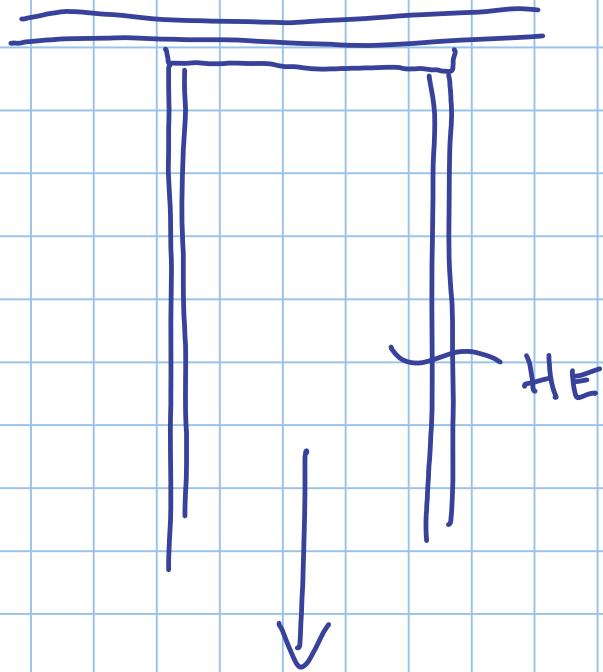
piatti
compresi



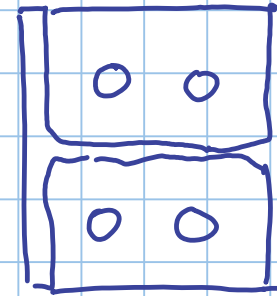
bullone



la resistenza a trazione
del bullone non dipende
della forza di serraggio



$$N_{Ed} = 600 \text{ kN}$$



4 bullw

$$N_{Rd} = 4 \times 0.9 A_{ns} \frac{f_{ub}}{\gamma_{m2}}$$

bullw 4.6

$$f_{ub} = 400 \text{ MPa}$$

$$N_{RA} = 4 \times 0.9 A_{us} \frac{f_{ub} \leftarrow 400 \text{ MPa}}{\gamma_{M2}} \geq N_{Ed} = 600 \text{ kN}$$

$$A_{us} \geq \frac{600 \times 10^3 \times 1.25}{4 \times 0.9 \times 400} = 520 \text{ mm}^2$$

$$4 \times 0.9 \times A_{us} \frac{f_{ub} \rightarrow \text{definiert}}{\gamma_{M2}} \geq 600$$

$$\downarrow$$

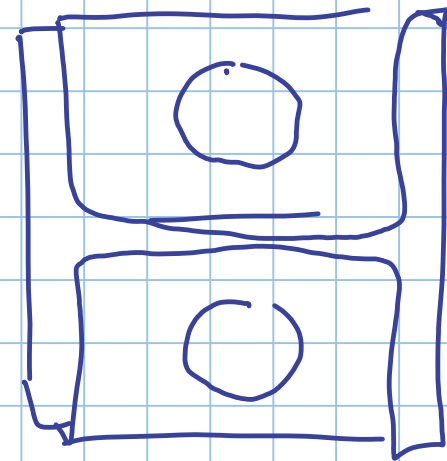
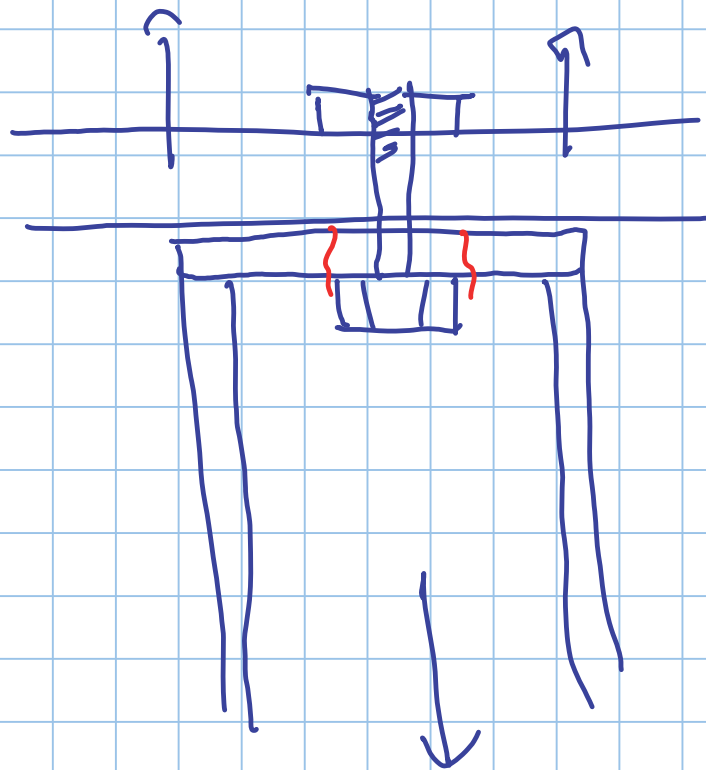
$$M20$$

$$245 \text{ mm}^2$$

$$f_{ub} \geq \frac{600 \times 10^3 \times 1.25}{4 \times 0.9 \times 245} = 850 \text{ MPa}$$

M10.9

In presenza di bulloni tesi
possibili rotture del piatto per PUNZONAMENTO

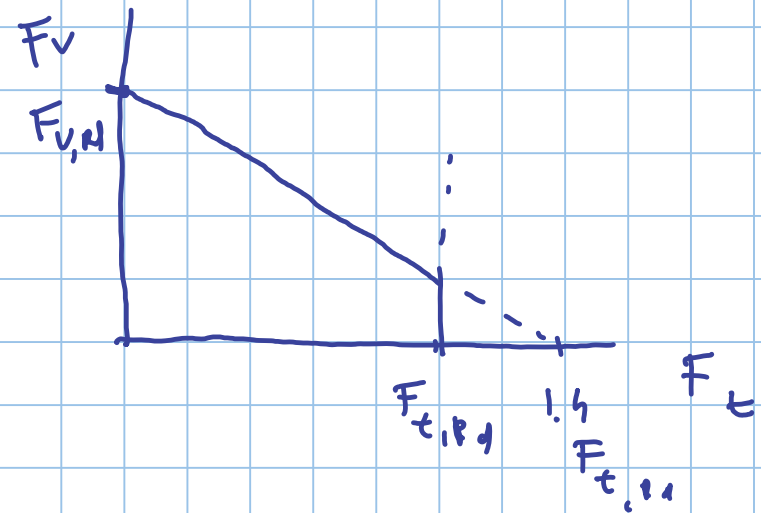


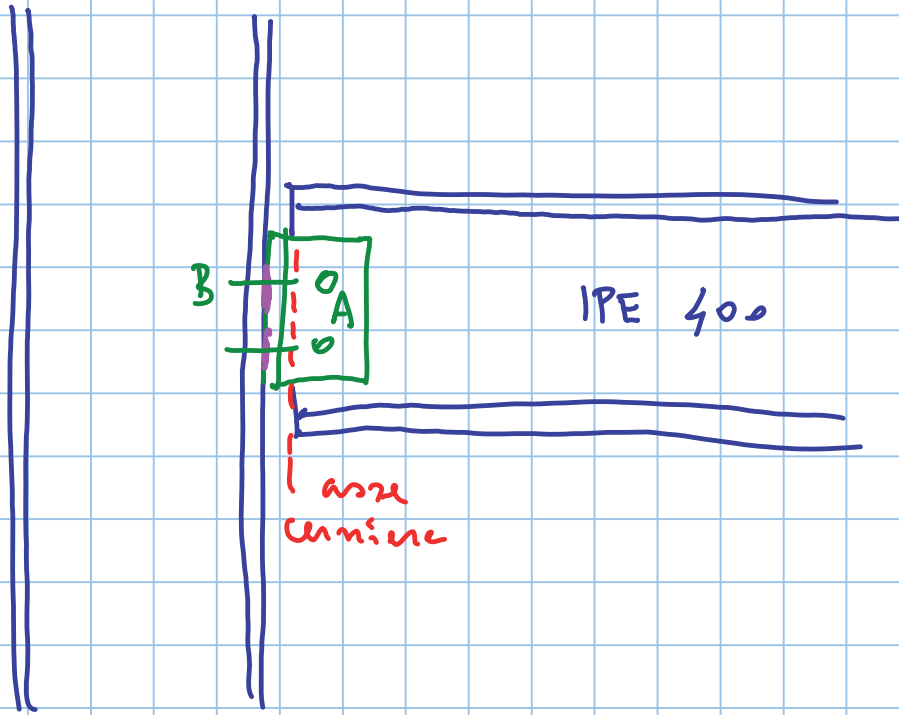
$$B_{r,R} = \pi d_m t \cdot 0.6 \frac{f_u}{\gamma_{M2}}$$

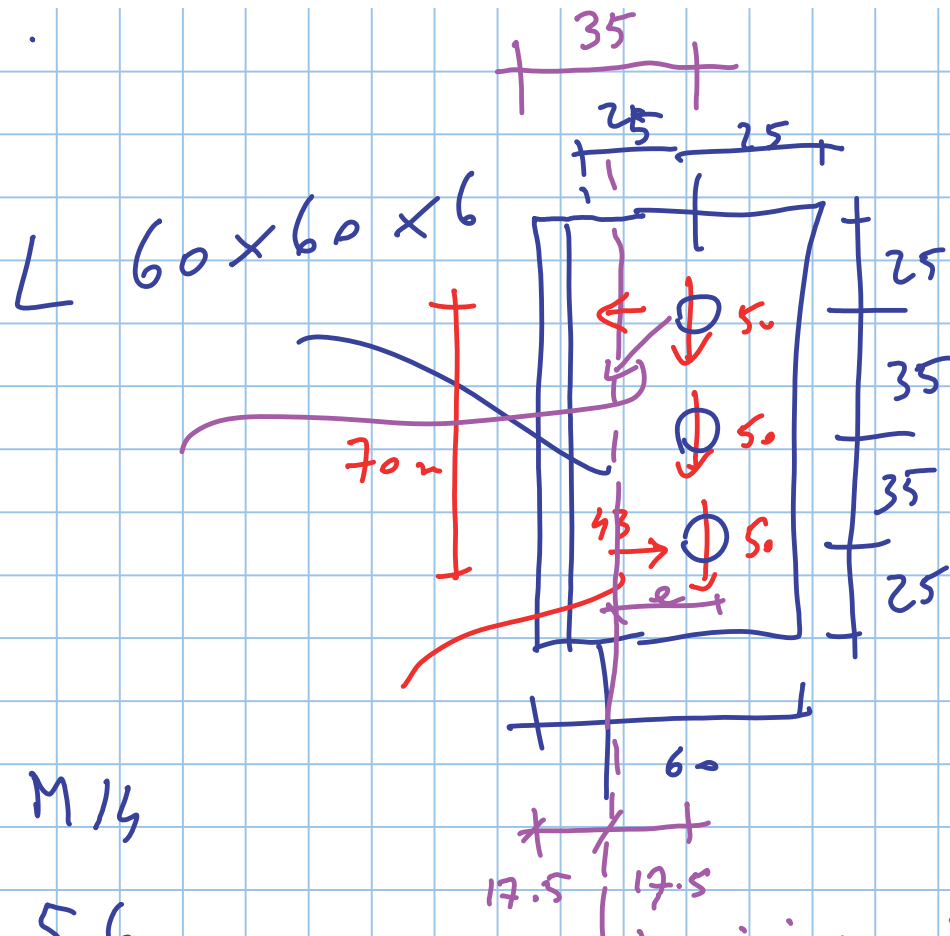
In presenza di Tagli ($F_{v,Ed}$) e Trazione ($F_{t,Ed}$) contempor.

$$\frac{F_{v,Ed}}{F_{v,Rd}} + \frac{F_{t,Ed}}{1.4 F_{t,Rd}} \leq 1$$

$$F_{t,Ed} \leq F_{t,Rd}$$







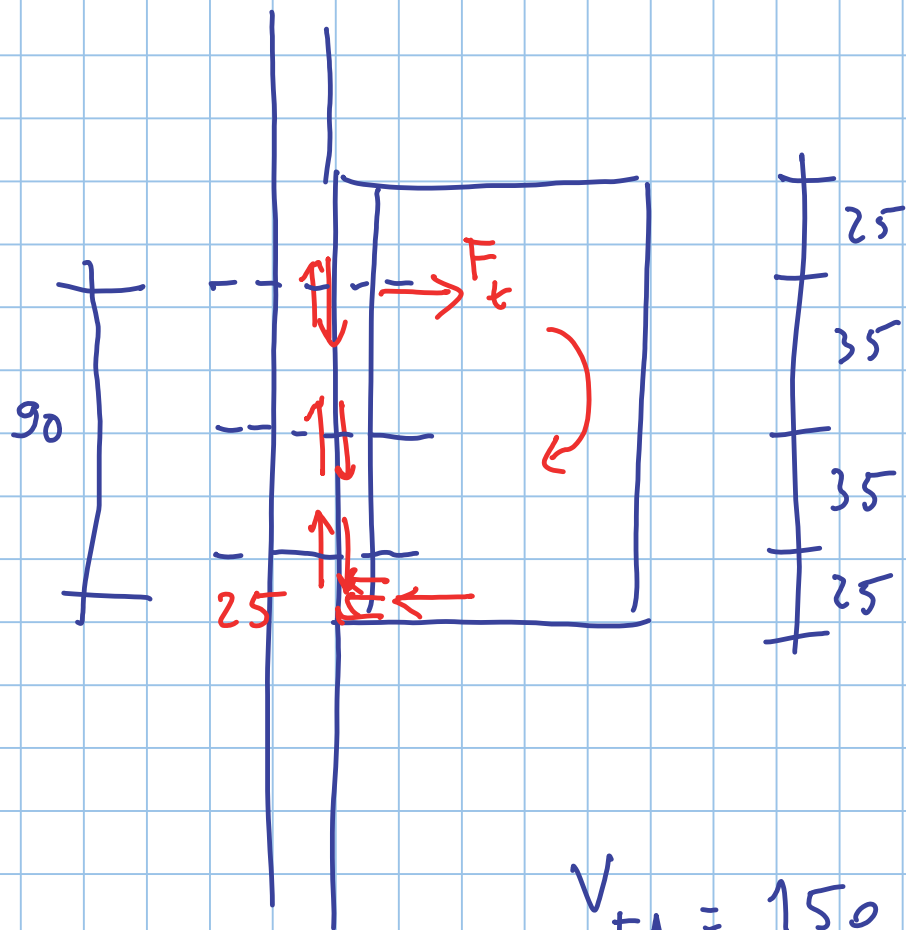
bulloni M 16

classe 5.6

posizione ideale
delle cerniere

$$V_{EA} = 150 \text{ kN}$$

$$M_{EA} = 150 \times 0.0175 = 2.625 \text{ kNm}$$



$$F_t = \frac{2.62}{0.090} \cdot \frac{1}{2} = 14.6 \text{ kN}$$

$$F_{v.Rd} = 37.0 \text{ kN} \quad \text{per connection}$$

$$F_{t.Rd} = 0.9 \times 115 \times \frac{500}{1.25} = 41.4 \text{ kN}$$

3 x 2 bolts $V_{Ed} = 150 \text{ kN}$
 $M_{Ed} = 2.62 \text{ kNm}$

$$\frac{25}{37.0} + \frac{14.6}{1.4 \times 41.4} = 0.928 < 1$$

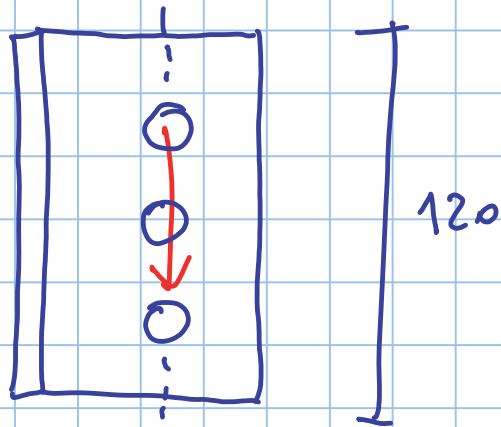
OK

2 L 60 x 60 x 6

Verifica dell'angolo a Taglio.

$$A = 120 \times 6 = 720 \text{ mm}^2$$

$$5275$$



azione interna

$$V_{RA} = A \frac{f_y / \sqrt{3}}{\gamma_m} = \frac{720 \times 275 / \sqrt{3}}{1.05} \times 10^{-3}$$

$$= 108.9 \text{ kN}$$

azione perate

$$A = 75 \times 6 = 450 \text{ mm}^2$$

x 2 ang.levi

$$V_{RA} = 2 \times 108.9 = 217.8 \text{ kN}$$

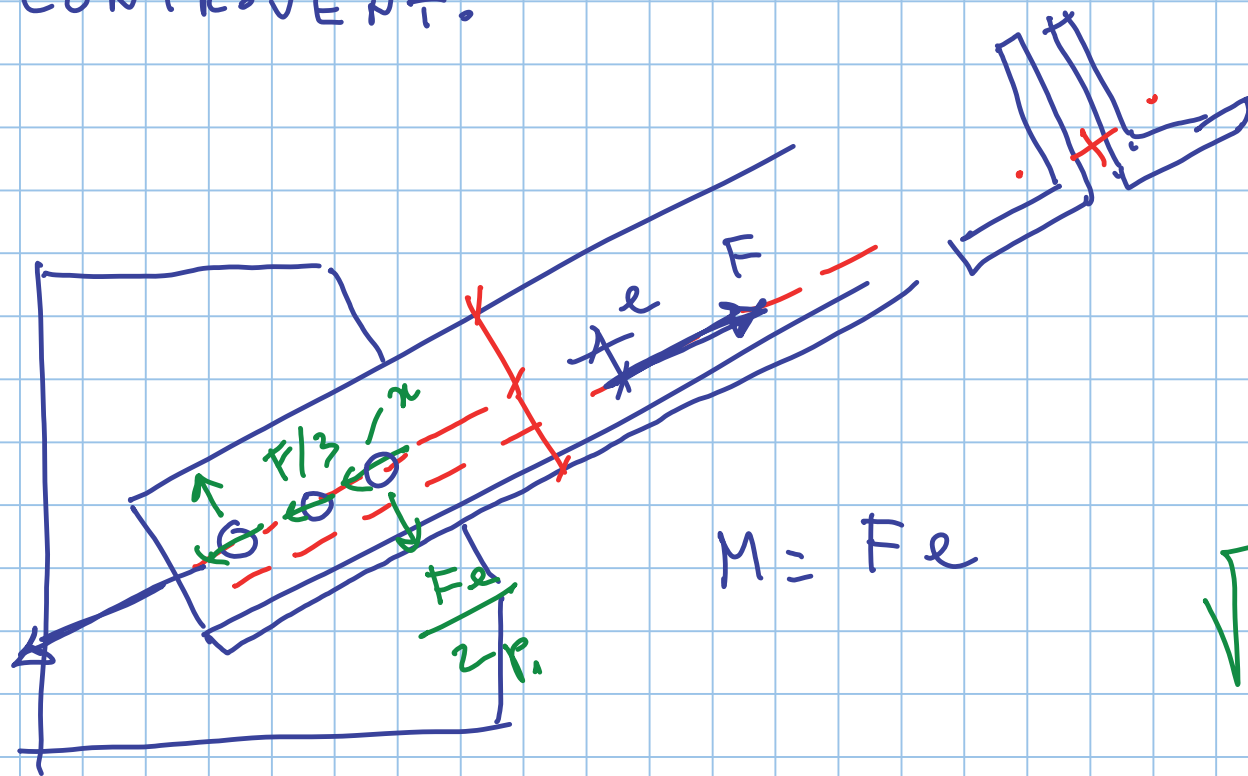
$$> V_{EA}$$

$$V_{n,RA} = 0.9 A_{n,R} \frac{f_u / \sqrt{3}}{\gamma_{m2}} = 0.9 \times 450 \times \frac{430 / \sqrt{3}}{1.25} \times 10^{-3} = 80.4 \text{ kN}$$

$$\times 2 = 160.8 \text{ kN}$$

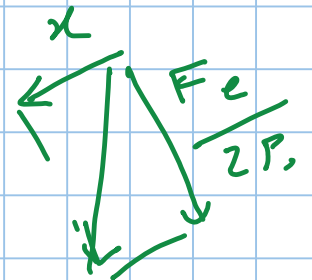
(OK)

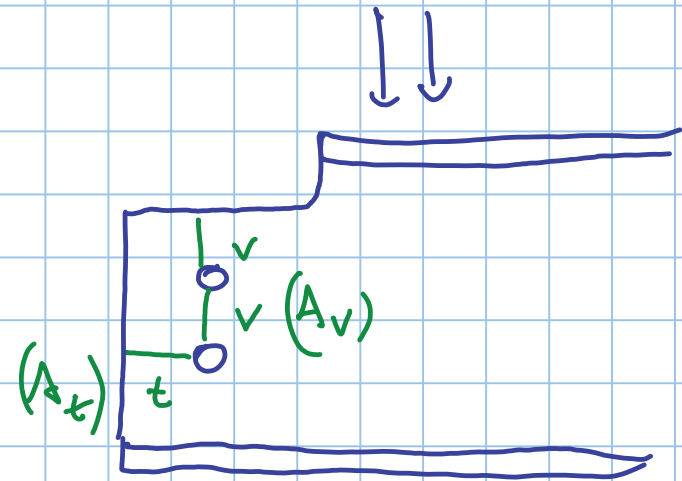
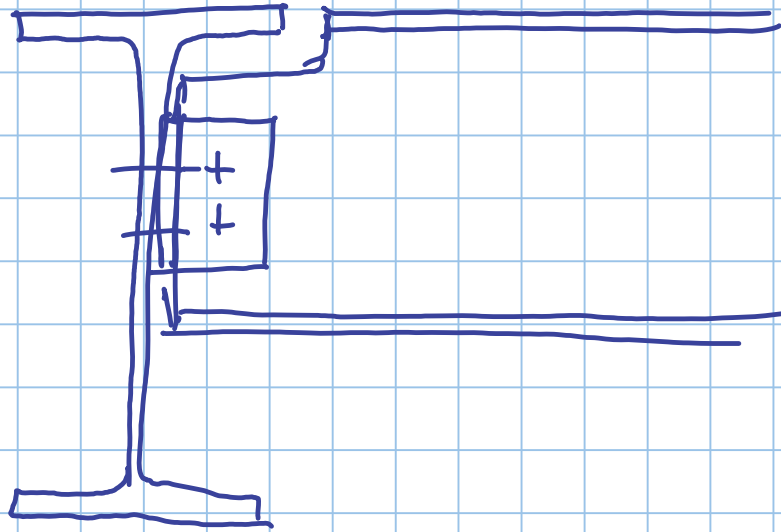
CONTRAVENT.



$$M = Fe$$

$$\sqrt{x^2 + \left(\frac{Fe}{2p_1}\right)^2} = F - 2x$$





BLOCK TEARING

$$F_{Rd} = A_t \frac{f_u}{\gamma_{m2}} + A_v \frac{f_y / \sqrt{3}}{\gamma_{m0}}$$