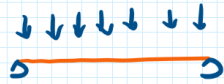
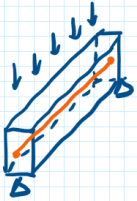


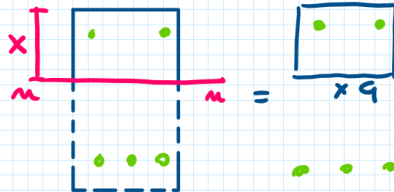
## NOTA SUL MODELLO NUMERICO



IN REALTÀ →



se le armature sono dissimetriche  
 $G \neq 0$



sez. reag. omog.  
 $G \neq 0$

⇒ Ipotesi  $G \neq 0$  ⇒ Risolvere lo schema ⇒

⇒ caratt. della sollecitazione

⇒ asse neutro ⇒  $G'$   
 $x'$

Itero il procedimento

finché  $G$  finale =  $G$  iniziali



NON È FATIGABILE



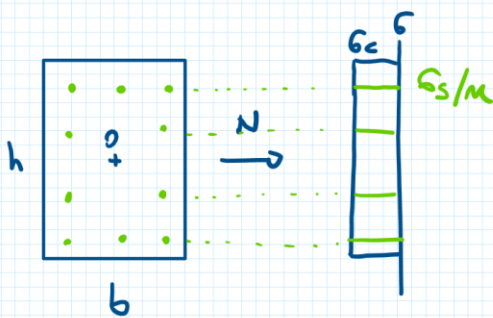
Baricentro "O" = centro geometrico

## SFORZO NORMALE

$N$   $\left\{ \begin{array}{l} \text{TRAZIONE } N^+ \\ \text{COMPRESSIONE } N^- \end{array} \right.$

$N^+$  TRAZIONE

I STADIO = Verifica a fessurazione



$$\sigma_{ct} \leq f_{ctk}$$

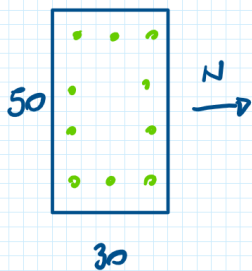
$$\sigma_{ct} = \frac{N}{A_{ci}}$$

$$A_{ci} = A_c + m A_{sTOT}$$

$$m = \frac{E_s}{E_c}$$

$$\sigma_s = m \sigma_c$$

## ESEMPIO



c 25/30 B 450 C

10  $\phi 14$

$N = 200 \text{ kN}$

I STADIO

$$\sigma_{ct} \leq f_{ctk}$$

$$f_{ctk} = 0.7 f_{ctm} = 0.7 \left( 0.3 \sqrt[3]{f_{cu}^2} \right) = 1.79 \text{ MPa}$$

$$\sigma_{ct} = \frac{N}{A_{ci}}$$

$$A_{ci} = 30 \times 50 + 6.35 \left( 10 \times 1.54 \right) = 1597.8 \text{ cm}^2$$

$$\sigma_{ct} = \frac{200}{1597.8} \times \frac{10^3}{10^2} = 1.25 \text{ MPa}$$

SEZ si È FESSURATA? NO!  $1.25 < 1.79$   
 $\sigma_{ct} < f_{ctk}$

OSSERVA:

$N_F$  DI FESSURAZIONE?

$$\sigma_{ct} = \frac{N_F}{A_{ci}} \Rightarrow N_F = A_{ci} \times f_{ctk} =$$

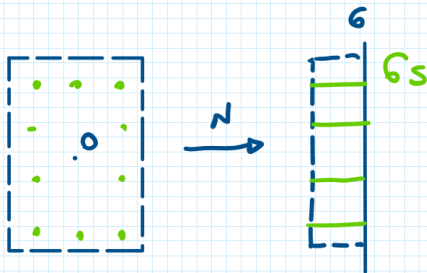
$$= 1597.8 \times 1.79 \cdot \frac{1}{10} = 286.0 \text{ kN}$$

$$\frac{\text{cm}^2}{\text{mm}^2} \times \frac{\text{N}}{\text{mm}^2} \times \frac{10^2}{10^3}$$

## II STADIO

verifiche sulle tensioni in esercizio

$$N > N_F$$

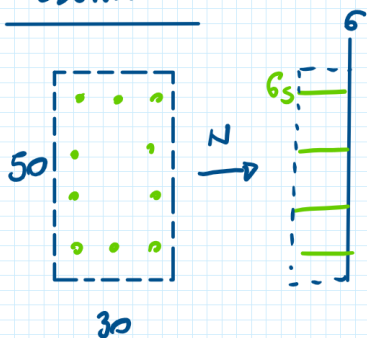


SEZ. REAG. DI SOLE ARMATURE

$$\sigma_s \leq 0.8 f_{yk}$$

$$\sigma_s = \frac{N}{A_{STOT}}$$

ESEMPIO



$$N = 400 \text{ kN}$$

COMB. CARICO RARA

II STADIO

$$\sigma_s = \frac{N}{A_{TOT}} = \frac{400}{15.4} \times 10 = 259.7 \text{ MPa}$$

$$A_{TOT} = 10 \times 1.54 = 15.4 \text{ cm}^2$$

$$\Rightarrow \sigma_s < 0.8 f_{yk} \\ \underline{\text{OK!}}$$

$$0.8 f_{yk} = 0.8 \cdot 450 = 360 \text{ MPa}$$

OSSERVA:

$$\bullet N = 235 \text{ kN} < N_F = 286 \text{ kN} \Rightarrow \text{I STADIO}$$

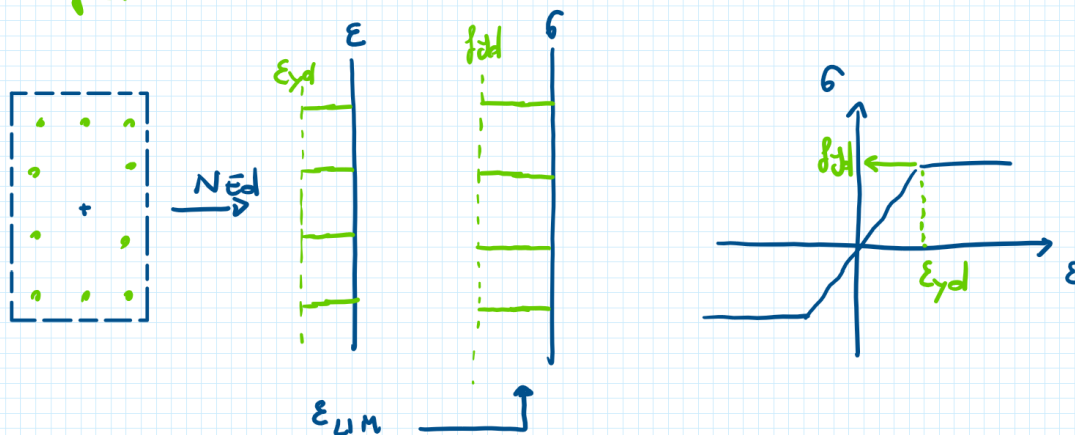
$$\sigma_s = m \sigma_c = m \frac{N}{A_{ci}} = 6.35 \times \left( \frac{235}{1597.3} \times 10 \right) = 11.33 \text{ MPa}$$

$$\bullet N = 287 \text{ kN} > N_F = 286 \text{ kN} \Rightarrow \text{II STADIO}$$

$$\sigma_s = \frac{N}{A_{TOT}} = \frac{287}{15.4} \times 10 = 186.4 \text{ MPa}$$

**III STADIO**

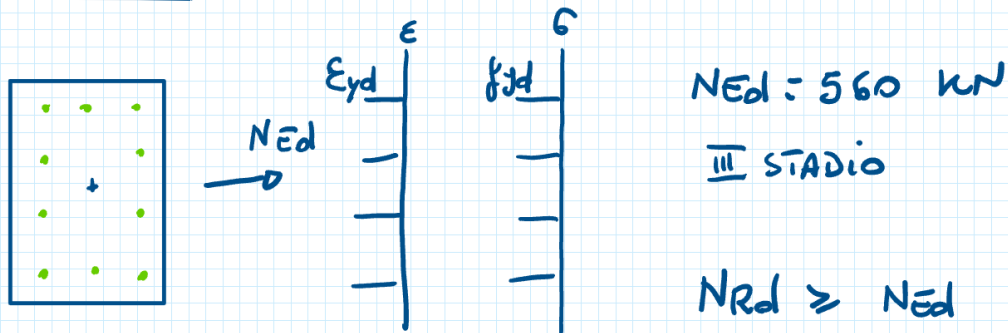
Verifiche SW



$$N_{Rd} \geq N_{Ed}$$

$$N_{Rd} = \int \sigma_s dA_s = f_{yd} A_{TOT} \quad \left( f_{yd} = \frac{f_{yk}}{\gamma_s} \right)$$

## ESEMPIO

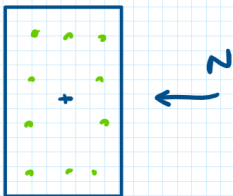


$$N_{Rd} = A_s f_{yd} = 10 \times 1.94 \times \frac{391.3}{10} = 602.6 \text{ kN}$$

Verifica?  $602.6 \geq 560 \text{ kN}$  **SI!**

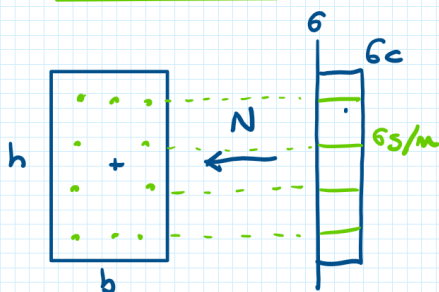
## $N^-$ COMPRESSIONE

### I STADIO



verif. a flessurazione non serve

### II STADIO



Verifiche sulle tensioni in esercizio

$$\sigma_c \leq 0.6 f_{ck} \quad (\text{COMB. RARA})$$

$$\sigma_c \leq 0.45 f_{ck} \quad (\text{COMB. QUASI PERM.})$$

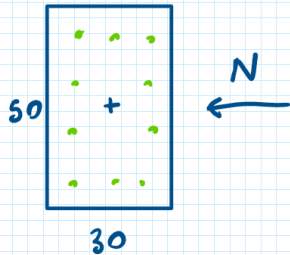
$$\sigma_s \leq 0.8 f_{yk} \quad (\text{COMB. RARA})$$

$$\sigma_c = \frac{N}{A_{ci}} ; \quad \sigma_s = \mu \sigma_c$$

$$A_{ci} = \text{secc. reag. mag.} = \underbrace{A_c}_{bh} + \underbrace{n}_{\text{per caucchi di lunghezza d'asta}} A_s$$

$n = 15$

### ESEMPIO



$$N = -1000 \text{ kN} \quad (\text{COMB. RARA})$$

II STADIO DI COMPORTAMENTO

$$\sigma_c \leq 0.6 f_{ck}$$

$$\sigma_c = \frac{N}{A_{ci}} = \frac{-1000}{1731} \times 10 = -5.8 \text{ MPa}$$

$$A_{ci} = 30 \times 50 + 15 (10 \times 1.56) = 1731 \text{ cm}^2$$

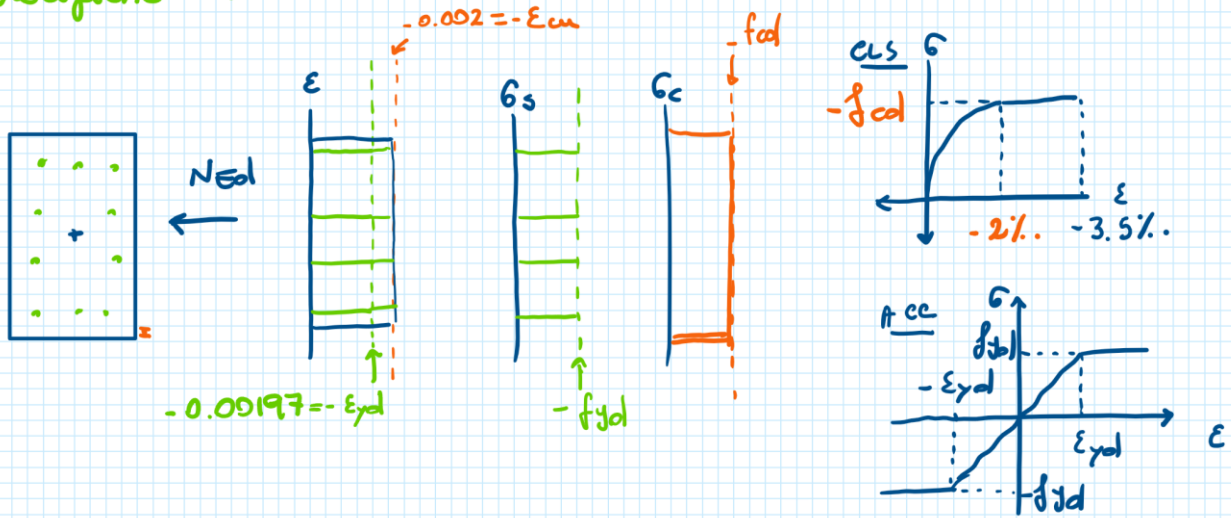
$$0.6 f_{ck} = 0.6 \times 25 = 15.0 \text{ MPa}$$

$$|\sigma_c| \leq 0.6 f_{ck} \Rightarrow \underline{\text{OK!}}$$

$$N_{\max} = A_{ci} \times 0.6 f_{ck} = 1731 \times \frac{15}{10} = 2596.5 \text{ kN}$$

### III STADIO

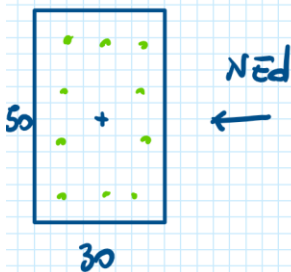
Verifiche SW



$$\begin{aligned}
 N_{Rd} &= \int \sigma dA = \int \sigma_c dA_c + \int \sigma_s dA_s = \\
 &= -f_{cd} A_c + \sum \sigma_s A_s = \\
 &= -f_{cd} A_c - f_{yd} A_{STOT}
 \end{aligned}$$

$$N_{Rd} = A_c f_{cd} + A_{s_{TOT}} f_{yd} \geq N_{Ed}$$

ESEMPIO



$$N_{Ed} = -1400 \text{ kN}$$

SW

$$N_{Rd} \geq N_{Ed}$$

$$\begin{aligned}
 N_{Rd} &= 30 \times 50 \times \frac{14.17}{10} + 10 \times 1.54 \times \frac{391.3}{10} = 2728.1 \text{ kN} \\
 &\quad \underbrace{\hspace{10em}}_{2125.5} \quad \underbrace{\hspace{10em}}_{602.6}
 \end{aligned}$$

$$2728.1 \geq |N_{Ed}| = 1400 \text{ kN} \quad \underline{\text{OK!}}$$

SLU e' più grande di SLE?

SLE



$$N = -2596 \text{ kN}$$

II STADIO

CAR. COMB. RARA

$g_k$   
 $q_k$

$$N \leq N_{\max \underline{II}} = -2596 \text{ kN}$$

SLU



$$N_{Ed} = -2596 \times (1.4) \rightarrow \left. \begin{array}{l} g_d = g_k \times 1.3 \\ q_d = q_k \times 1.5 \end{array} \right\} (g_k + q_k) \times 1.4$$

$$N_{Ed} = -3634.4 \text{ kN}$$

$$N_{Rd} = 2728.1 \text{ kN} \leq |3634.4|$$

SLU non  
e' soddisfatto