

Sezione HEB200

Acciaio S235

Sezione di classe 1

$F_1 = 500 \text{ kN}$

$F_2 = 80 \text{ kN}$

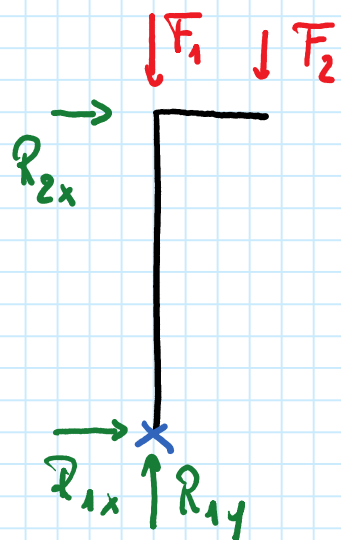
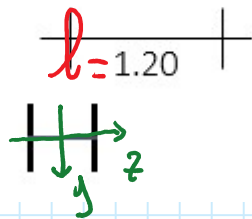
$$A = 78,1 \text{ cm}^2$$

$$i_y = 8,54 \text{ cm}$$

$$i_z = 5,07 \text{ cm}$$

$$I_y = 5696 \text{ cm}^4$$

$$W_{pl,y} = 642,5 \text{ cm}^3$$



Equilibrio alle rotazioni attorno al punto x

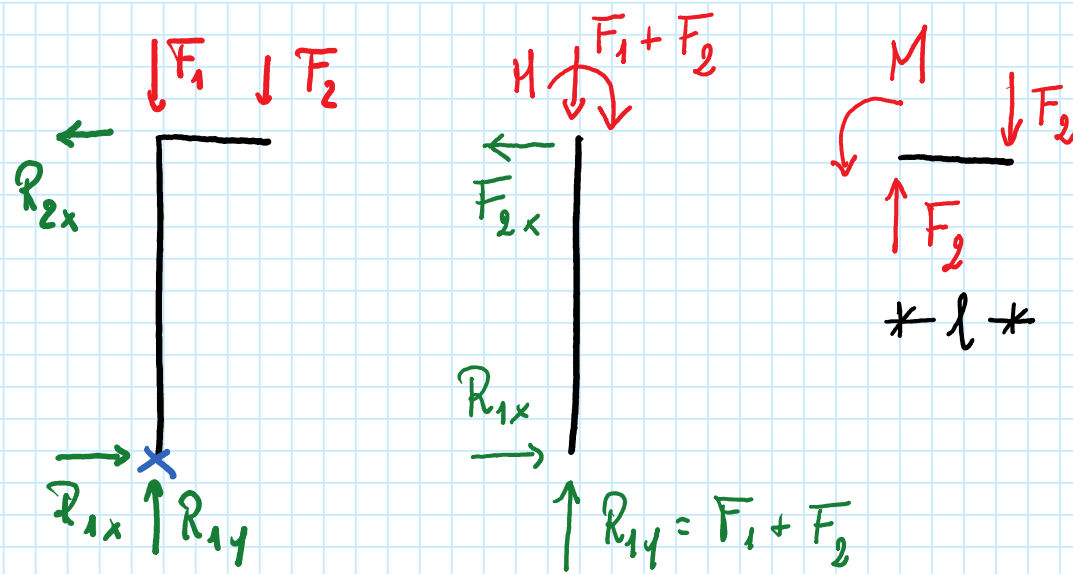
$$-R_{2x} H - F_2 l = 0 \Rightarrow R_{2x} = -F_2 \frac{l}{H} = -\frac{80}{3} \times 1,2 = -32 \text{ kN}$$

Equilibrio alle traslazioni x

$$R_{1x} = -R_{2x} = 32 \text{ kN}$$

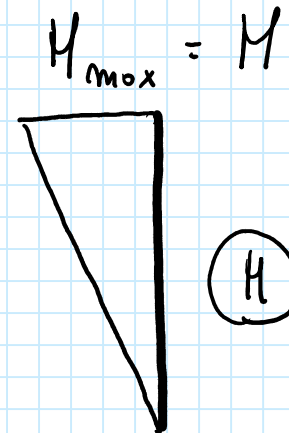
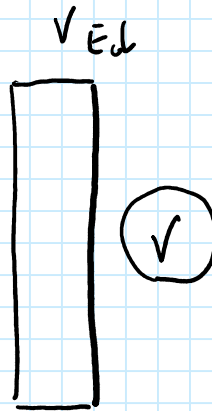
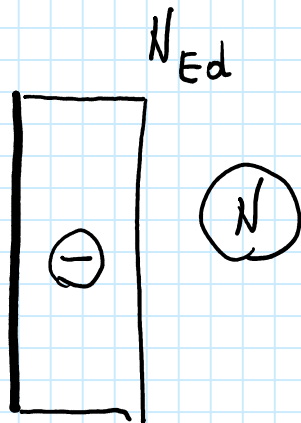
Equilibrio alle traslazioni y

$$R_{1y} = F_1 + F_2 = 500 + 80 = 580 \text{ kN}$$



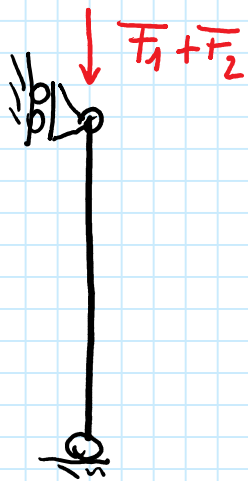
$$M = F_2 l = 80 \times 1,2$$

$$= 96,0 \text{ kNm}$$



$$N_{Ed} = -R_{1y} = -580,0 \text{ kN} \quad (\text{compression})$$

$$H_{max} = 96,0 \text{ kNm}$$



(N)

(M)



$$N = -(F_1 + F_2)$$

$$= -580 \text{ kN}$$

$$M_{\max} = F_2 l = 80 \times 1,2 = 96,0 \text{ kNm}$$

$$l_{0y} = H = 3,0 \text{ m}$$

$$l_{0z} = H = 3,0 \text{ m}$$

$$\frac{N_{Ed}}{N_{b,y,Rd}} + k_{yy} \frac{M_{y,Ed}}{M_{y,Rd}} + k_{yz} \frac{M_{z,Ed}}{M_{z,Rd}} \leq 1$$

$$\frac{N_{Ed}}{N_{b,z,Rd}} + k_{zy} \frac{M_{y,Ed}}{M_{y,Rd}} + k_{zz} \frac{M_{z,Ed}}{M_{z,Rd}} \leq 1$$

Profilo =	HEB 200	Acciaio =	S235	$f_{yk} =$	235 MPa		
Doppio profilo	NO	GammaM1=	1.05	$f_{uk} =$	360 MPa		
Distanza =	cm			$E_s =$	210000 MPa		
Area =	78.1 cm ²						
$\rho_y =$	8.54 cm	$I_{oy} =$	300.0 cm	$\lambda_y =$	35.1		
$\rho_z =$	5.06 cm	$I_{oz} =$	300.0 cm	$\lambda_z =$	59.2	$\lambda_{z\ eq} =$	
$\rho_{min\ SP} =$	cm	$I_{o\ SP} =$	75.0 cm	$\lambda_{min\ SP} =$			
Curva =	b	$\alpha_y =$	0.34	$N_{cr,y} =$	13104.1 kN	$\lambda_{Sy} =$	0.3742
Curva =	c	$\alpha_z =$	0.49	$N_{cr,z} =$	4608.0 kN	$\lambda_{Sz} =$	0.6311
$\phi_y =$	0.5997	$\chi_y =$	0.9362	$N_{b,Rd,y} =$	1636.38 kN		
$\phi_z =$	0.8048	$\chi_z =$	0.7668	$N_{b,Rd,z} =$	1340.33 kN		
		$\chi_{min} =$	0.7668	$N_{b,Rd} =$	1340.33 kN		

$$M_{y,Rd} = M_{pl,y,Rd} = W_{pl,y} \frac{f_y}{\gamma_{M1}} = 642.5 \times \frac{235}{1.05} \times \frac{1}{10^3} = 143.8 \text{ kNm}$$

$$k_{yy} = C_{my} \left(1 + (\bar{\lambda}_y - 0.2) \frac{N_{Ed}}{N_{b,Rd,y}} \right) \leq C_{my} \left(1 + 0.8 \frac{N_{Ed}}{N_{b,Rd,y}} \right)$$

$$= 0.6 \times \left[1 + (0.3742 - 0.2) \times \frac{580}{1636.4} \right] = \underline{0.6370}$$

$$\leq 0.6 \times \left(1 + 0.8 \times \frac{580}{1636.4} \right) = \cancel{0.9401}$$

$$c_{my} = 0.6 + 0.4 \psi \geq 0.4 \Rightarrow c_{my} = 0.6 + 0.4 \times 0 = \underline{0.6} > 0.4$$



$$\psi = \frac{0}{H} = 0$$

$$\frac{N_{Ed}}{N_{b,y,Rd}} + k_{yy} \frac{M_{y,Ed}}{M_{y,Rd}} + k_{yz} \frac{M_{z,Ed}}{M_{z,Rd}} \leq 1$$

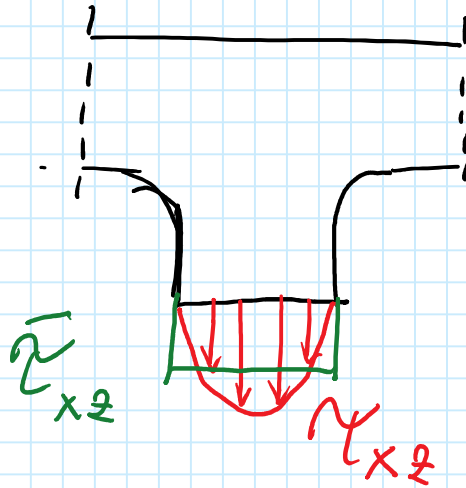
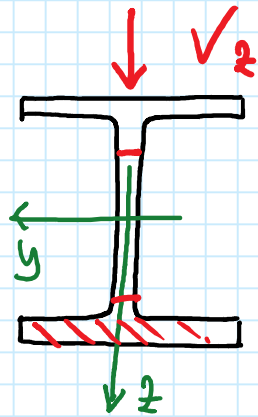
$$\frac{N_{Ed}}{N_{b,z,Rd}} + k_{zy} \frac{M_{y,Ed}}{M_{y,Rd}} + k_{zz} \frac{M_{z,Ed}}{M_{z,Rd}} \leq 1$$

$$\frac{580}{1636,8} + 0,6370 \times \frac{96,0}{143,8} = 0,3543 + 0,4253 = 0,7796 < 1$$

$$\frac{580}{1341,6} = 0,4323 < 1$$

OK!

Taglio



Formule di Jourawsky

$$\tau_{xz} = \frac{V_z S_y}{I_y b}$$

Stiamo assumendo

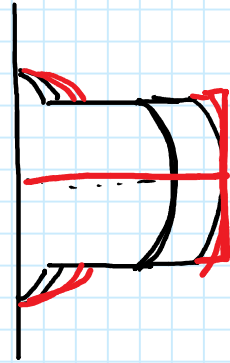
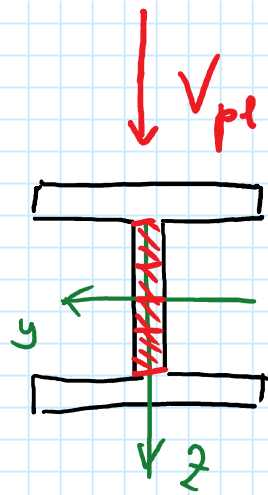
$$\tau_{xz} = \bar{\tau}_{xz}$$

V_z : Taglio agente

I_y : Momento d'inerzia dell'intera sezione rispetto a y

b : lunghezza delle code

S_y : Momento statico rispetto ad y delle "parti di sezione sotto (o sopra) la corda"



$$\tau_{max} = \frac{f_y}{\sqrt{3}}$$

Non posso utilizzarle
quando le sezioni si
plasticizzano

~~$$\tau_{x2} = \frac{V_z S_y}{I_y b}$$~~

$$\sigma_{id} = \sqrt{\sigma^2 + 3\tau^2} = \sqrt{0 + 3\tau_{max}^2} = \underbrace{\sqrt{3} \tau_{max}} = f_y$$

$$\tau_{max} = \tau_y = \frac{f_y}{\sqrt{3}}$$

limite elastico

Oltre il limite elastico...

$$V_{pl} = t_w h_w \frac{f_y}{\sqrt{3}}$$

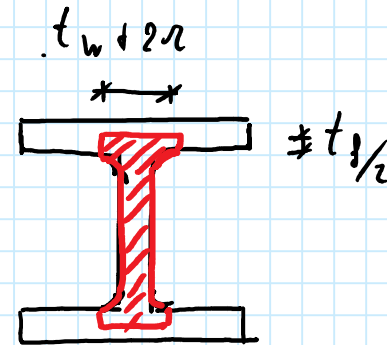
taglio che produce la plasticizzazione di
tutte l'anima.

Formule delle NTC 18 per il calcolo delle resistenze a taglio di sezioni a doppio T sollecitate nel piano dell'anima

$$V_{e,Rd} = V_{pl,Rd} = \frac{A_v f_y}{\sqrt{3} \gamma_{M0}}$$

A_v = area resistente a taglio

$$A_v = A - 2bt_f + (t_w + 2z)t_f$$



Le formule per il calcolo di $V_{e,Rd}$ è valide anche per altri tipi di sezione ma cambia il calcolo di A_v

	Forma della sezione	Direzione di V	A_v
Profilati ad I o H		Z-Z (anima)	$A - 2bt_f + (t_w + 2r)t_f$
		y-y (ali)	$A - \sum (h_w t_w)$
Profilati a C o U		Z-Z (anima)	$A - 2bt_f + (t_w + r)t_f$
Profilati a T		Z-Z (anima)	$A - bt_f + (t_w + 2r)\frac{t_f}{2}$ (EC3) $0.9(A - bt_f)$ (NTC08)
Sezioni saldate a T		Z-Z (anima)	$t_w \left(h - \frac{t_f}{2} \right)$
Scatolari		Z-Z (anime)	$\frac{Ah}{b+h}$
		y-y (basi)	$\frac{Ab}{b+h}$
Tubolari		--	$\frac{2A}{\pi}$

A = area nominale della sezione trasversale

In sintesi...

1. Se ho A_v + ebbi 1 e 2

$$V_{Ed} \leq V_{c,Rd} : V_{pl,Rd} = \frac{A_v f_y}{\sqrt{3} \gamma_{M0}}$$

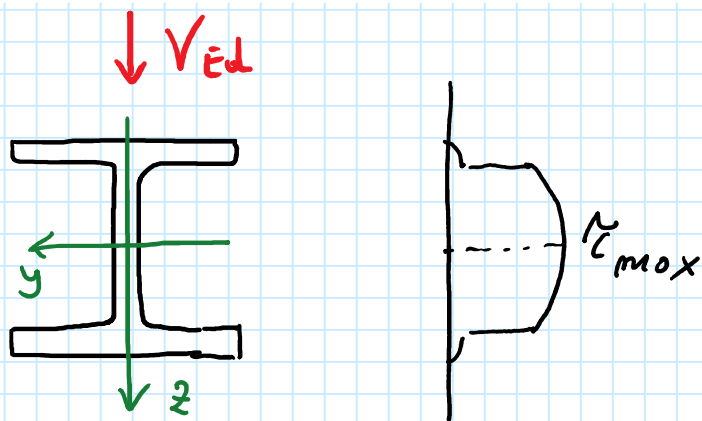
2. altrimenti: (verificare ebbi 3)

Calcolo τ_{max}

con

Journaw, K_T

$$\tau_{max} \leq \frac{f_y}{\sqrt{3} \gamma_{M0}}$$



IPE 270

S 235

$$V_{Ed} = 200 \text{ kN}$$

Comportamento elastico
(classe 3)

$$d. \tau_{max} = \frac{V_{Ed} S_y}{I_y b}$$

$$b = t_w ; S_y = \frac{W_{pl,y}}{2}$$

$$2. \tau_{max} \leq \frac{f_y}{\sqrt{3} \gamma_{M0}}$$

	G kg/m	h mm	b mm	t _w mm	t _f mm	r mm	A cm ²	h _i mm	d mm	Ø	P _{min} mm	P _{max} mm	A _L m ² /m	A _G m ² /t
IPE A 270*	30.7	267	135	5.5	8.7	15	39.15	249.6	219.6	M16	70	72	1.037	33.75
IPE 270	36.1	270	135	6.6	10.2	15	45.95	249.6	219.6	M16	72	72	1.041	28.86

	G kg/m	I _y cm ⁴	W _{el,y} cm ³	W _{pl,y} † cm ³	i _y cm	A _{vz} cm ²	I _z cm ⁴	W _{el,z} cm ³	W _{pl,z} † cm ³	i _z cm	s _s mm	I _t cm ⁴	I _w × 10 ⁻³ cm ⁶	S235	S355	S460	S235	S355	S460
IPE A 270	30.7	4917	368.3	412.5	11.21	18.75	358.0	53.03	82.34	3.02	40.47	10.30	59.51	1	1	-	3	4	-
IPE 270	36.1	5790	428.9	484.0	11.23	22.14	419.9	62.20	96.95	3.02	44.57	15.94	70.58	1	1	-	2	3	-

$$b = t_w = 6.6 \text{ mm}$$

$$I_y = 5790 \text{ cm}^4$$

$$S_y = \frac{W_{\text{ply}}}{2} = \frac{484}{2} = 242 \text{ cm}^3$$

$$\sigma_{\text{max}} = \frac{200 \times 242}{5790 \times 6.6} \times \frac{10^3 \times 10^3}{10^6} = 126.7 \text{ MPa}$$

$$\sigma_{\text{max}} = 126.7 \text{ MPa} \leq \frac{f_y}{\sqrt{3} \gamma_{M0}} = \frac{235}{\sqrt{3} \times 1.05} = 129.2 \text{ MPa}$$

OK!