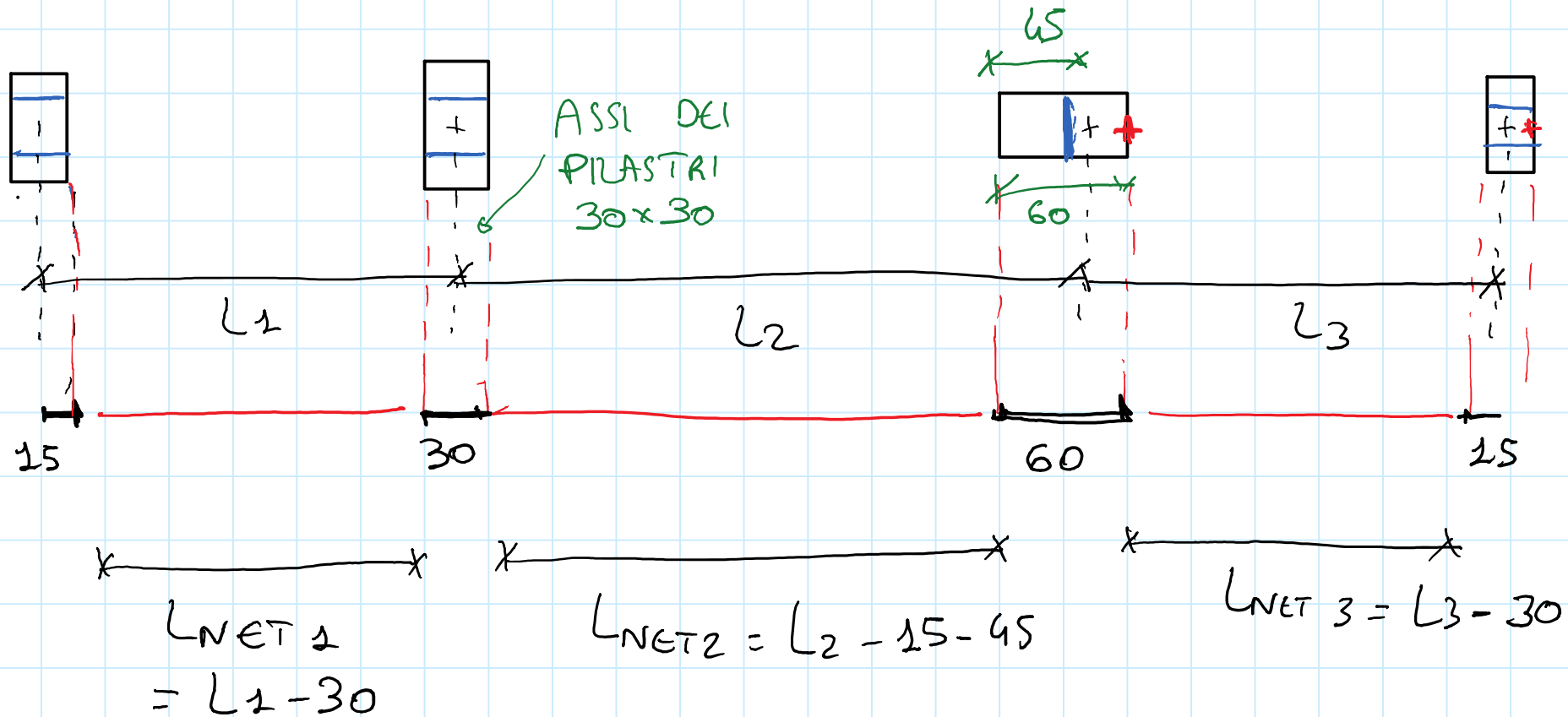
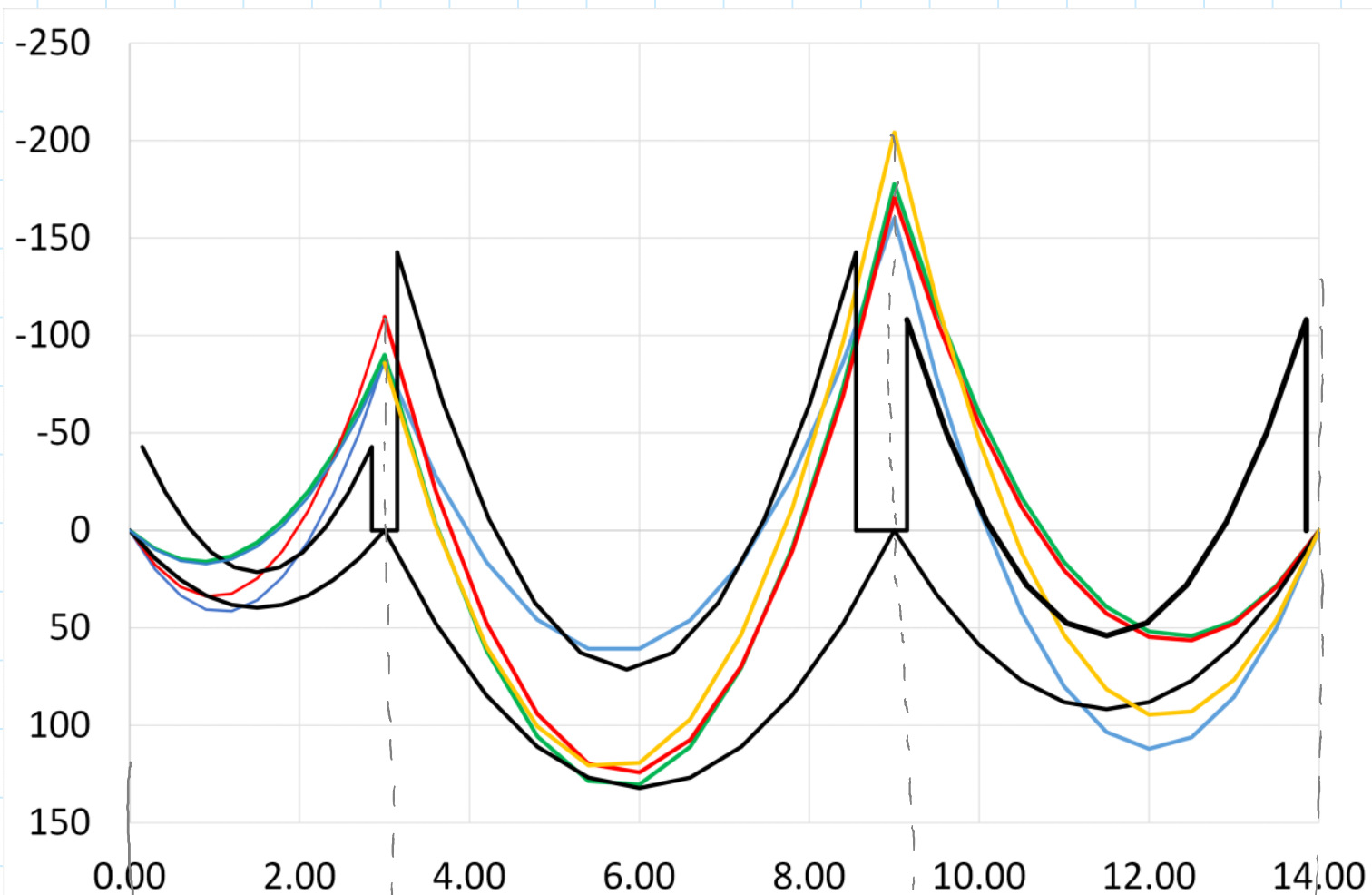


# UTILIZZO DI MOCAD PER LO SCHEMA LIMITE

DEFINISCO UN NUMERO DI ASTE FITTIZIAMENTE MAGGIORE  
DELLE CAMPATE PRESENTI



# DIAGRAMMI DEI MOMENTI OTTENUTI

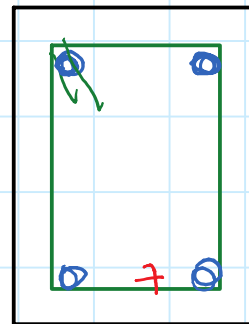


ASSI PILASTRI  
30x30

# ARMATURA MINIMA

NUMERO DI BARRE:

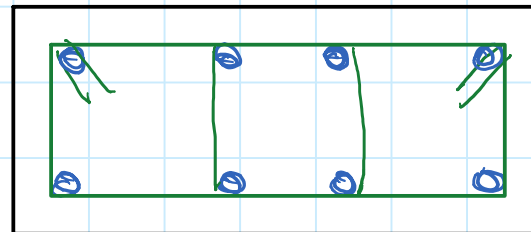
TRAVE EMERGENTE



2 BARRE SUPERIORI

2 BARRE INFERIORI  
(MEGLIO 3 PER  
RIDURRE AMPIEZZA  
FESSURE)

TRAVE A SPESSORE



ALMENO  
4 BARRE SUP;  
2 BARRE INF,

# ARMATURA MINIMA NTC 18 (ARMATURA TESA)

1.  $A_s \geq 0.13\% \cdot b \cdot d$

2.  $A_s \geq 0.26 \frac{f_{ctm}}{f_{yk}} b \cdot d$  (EVITARE SNERVAMENTO DOPO FESSURAZIONE CLS)

PER C30/37  $\rightarrow f_{ctm} = 0.3 \sqrt[3]{f_{ck}^2} = 2.89 \text{ MPa} \rightarrow$

$$A_s \geq 0.26 \times \frac{2.89}{450} b \cdot d = 0.167\% \cdot b \cdot d$$

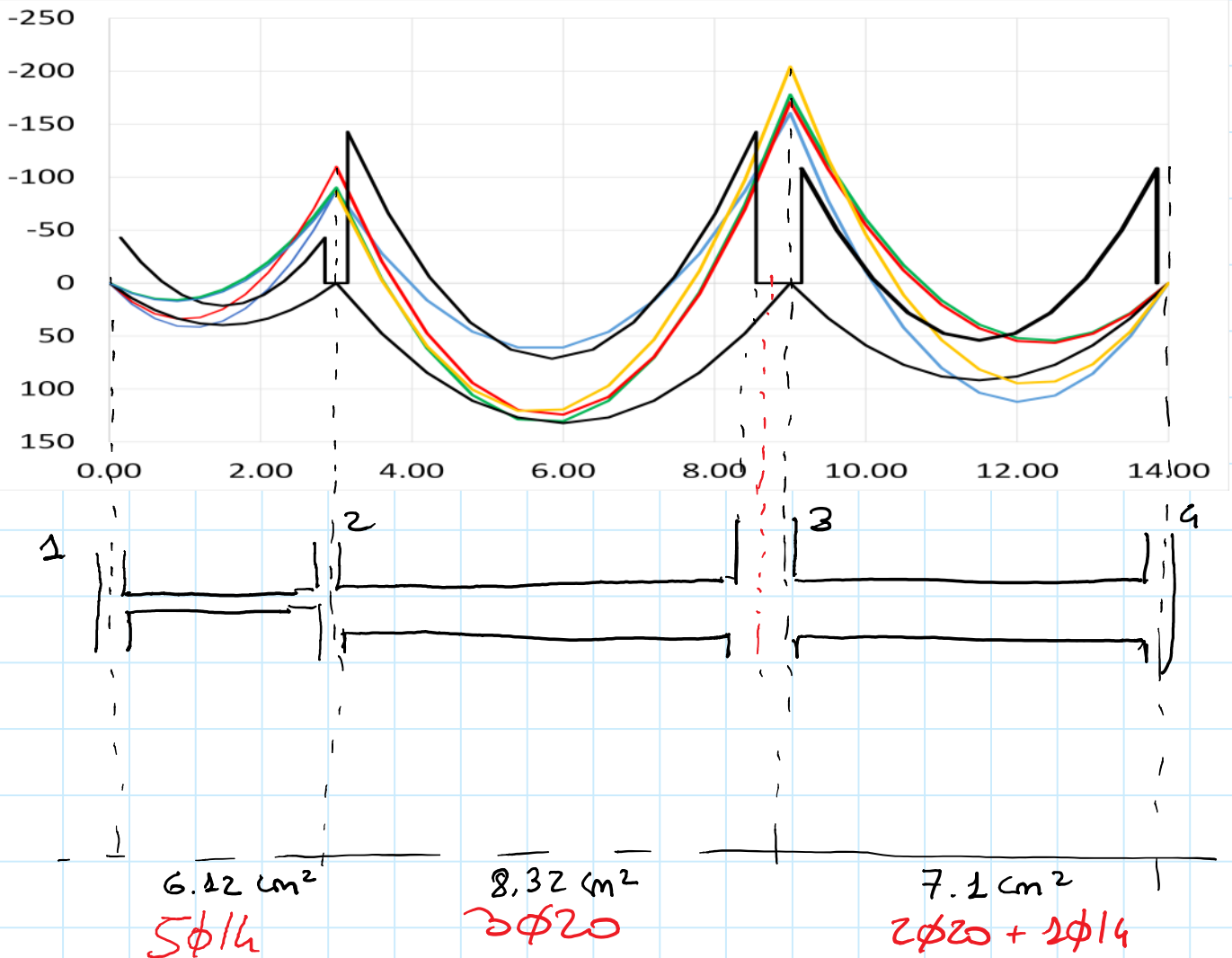
PER TRAVE 30x50  $\Rightarrow$

$$A_s \geq 0.167\% \cdot 30 \times 45 \text{ cm}^2 = 2.26 \text{ cm}^2 \rightarrow 2\phi 14$$

PER TRAVE 100x23  $\rightarrow (d = 19 \text{ cm}) \rightarrow$

$$A_s \geq 0.167\% \cdot 100 \times 19 \text{ cm}^2 = 3.18 \text{ cm}^2 \rightarrow 4\phi 14$$

# PROGETTO ARMATURA INFERIORE



$$A_s = \frac{M}{0.9 d f_{yd}}$$

CAMPATA 1-2

$$M = 41 \text{ kNm}$$

$$d = 23 - 4 = 19 \text{ cm}$$

$$A_s = \frac{41 \text{ kNm} \times 10}{0.9 \times 0.19 \text{ m} \times 391.3 \text{ N/mm}^2} = 6.12 \text{ cm}^2$$

CAMPATA 2-3

$$M = 132 \text{ kNm}$$

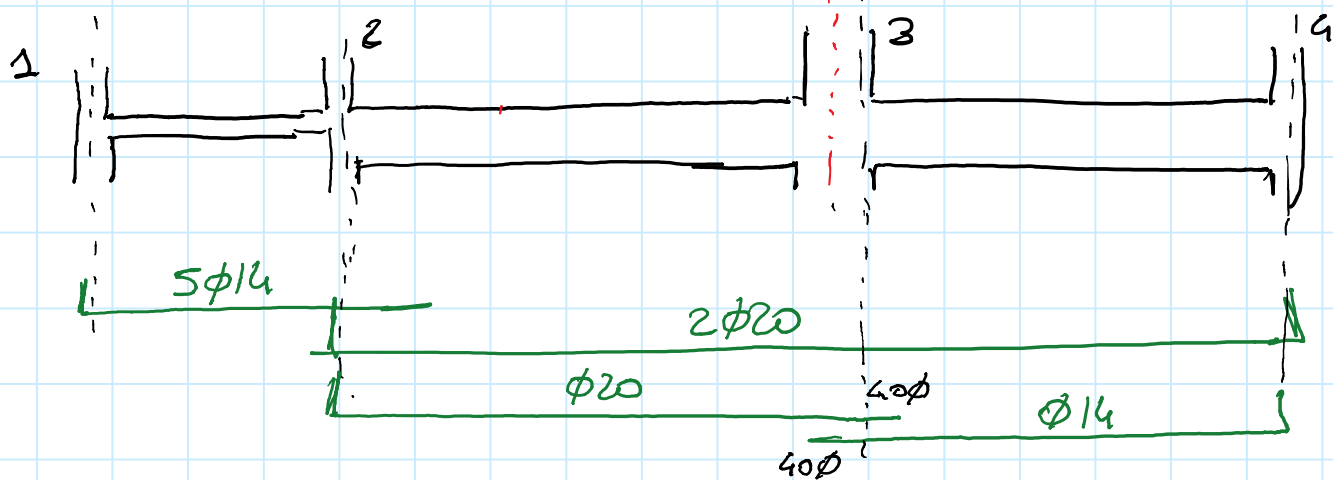
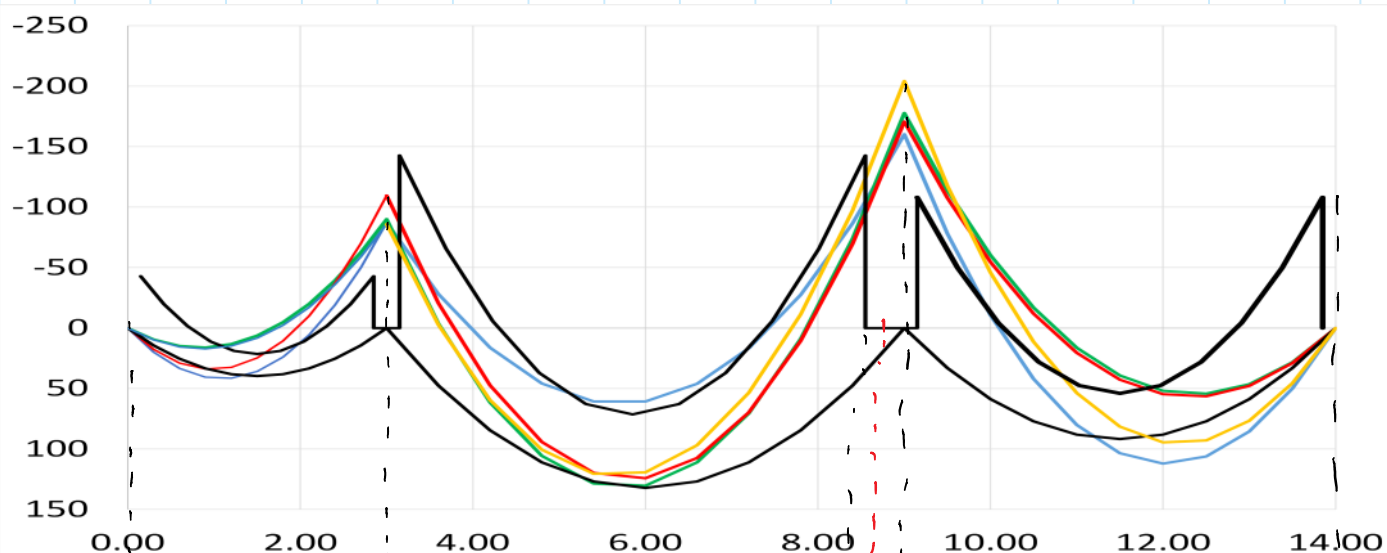
$$d = 50 - 5 = 45 \text{ cm}$$

$$A_s = \frac{132 \times 10}{0.9 \times 0.45 \times 391.3} = 8.32 \text{ cm}^2$$

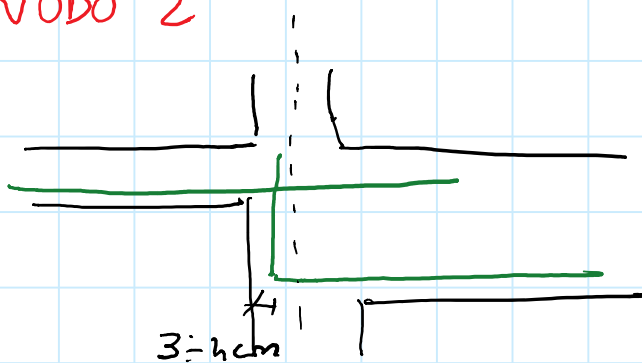
CAMPATA 3-4

$$M = 112 \text{ kNm} \rightarrow A_s = 7.1 \text{ cm}^2$$

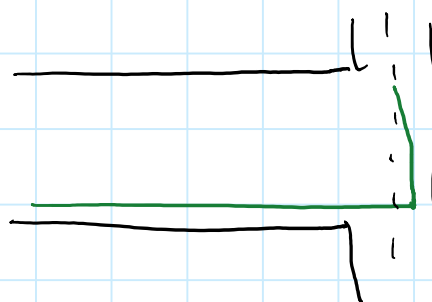
# DISPOSIZIONE ARMATURA INFERIORE



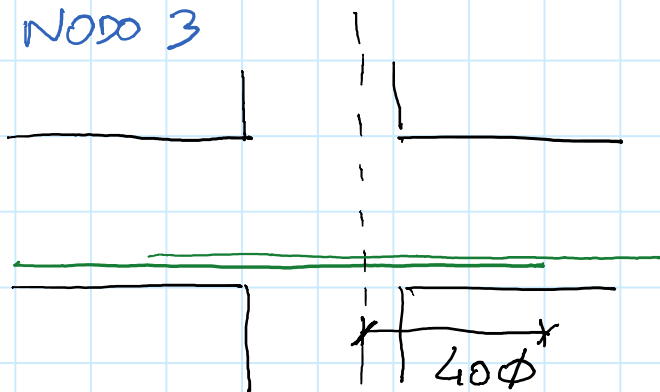
NODO 2



NODO 4



NODO 3

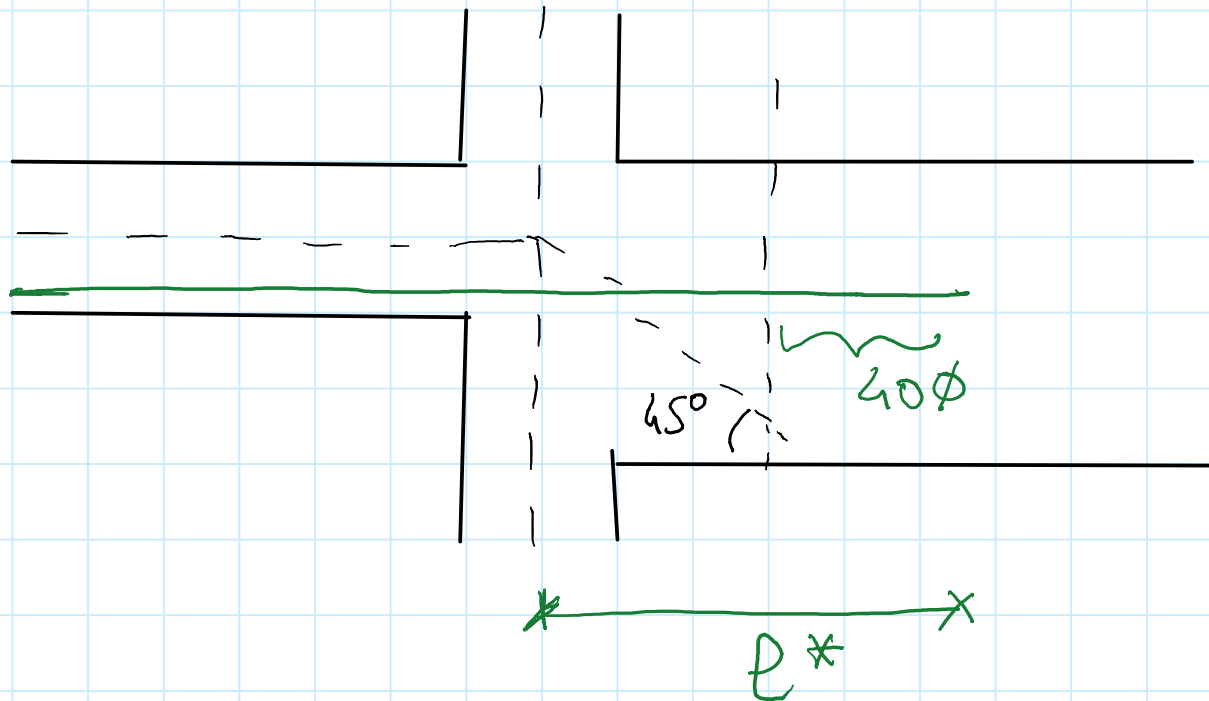


NODO 1



## ZONA DI PERTINENZA DELLA TRAVE A SPESSORE

1. PROLUNGO ASSE TRAVE A SPESSORE FINO AD INCONTRARE ASSE PIASTRO
2. CONSIDERO DIFFUSIONE DELLE TENSIONI A  $45^\circ$



MOMENTO RESISTENTE ARMATURE INFERIORI

TRAVE A SPESSORE

$$M_{rd} = A_s \times 0.9d \sigma_{yd}$$

$$M_{rd} (1\phi 14) = 1.54 \text{ cm}^2 \times 0.9 \times 0.19 \text{ m} \times 391.3 \frac{\text{N}}{\text{mm}^2} \frac{1}{20} = 10.30 \text{ kNm}$$

$$\rightarrow M_{rd} (5\phi 14) = 5 \times 10.30 = 51.5 \text{ kNm}$$

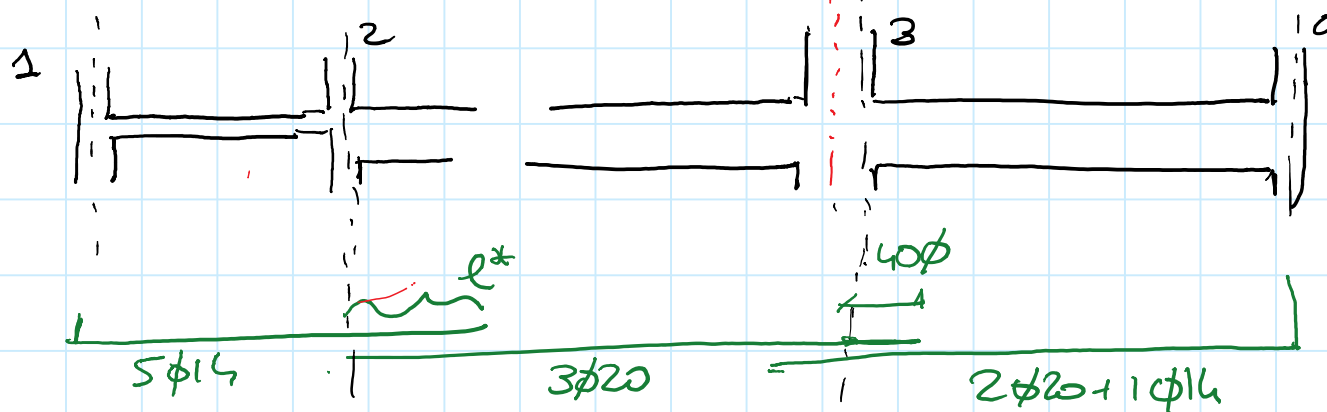
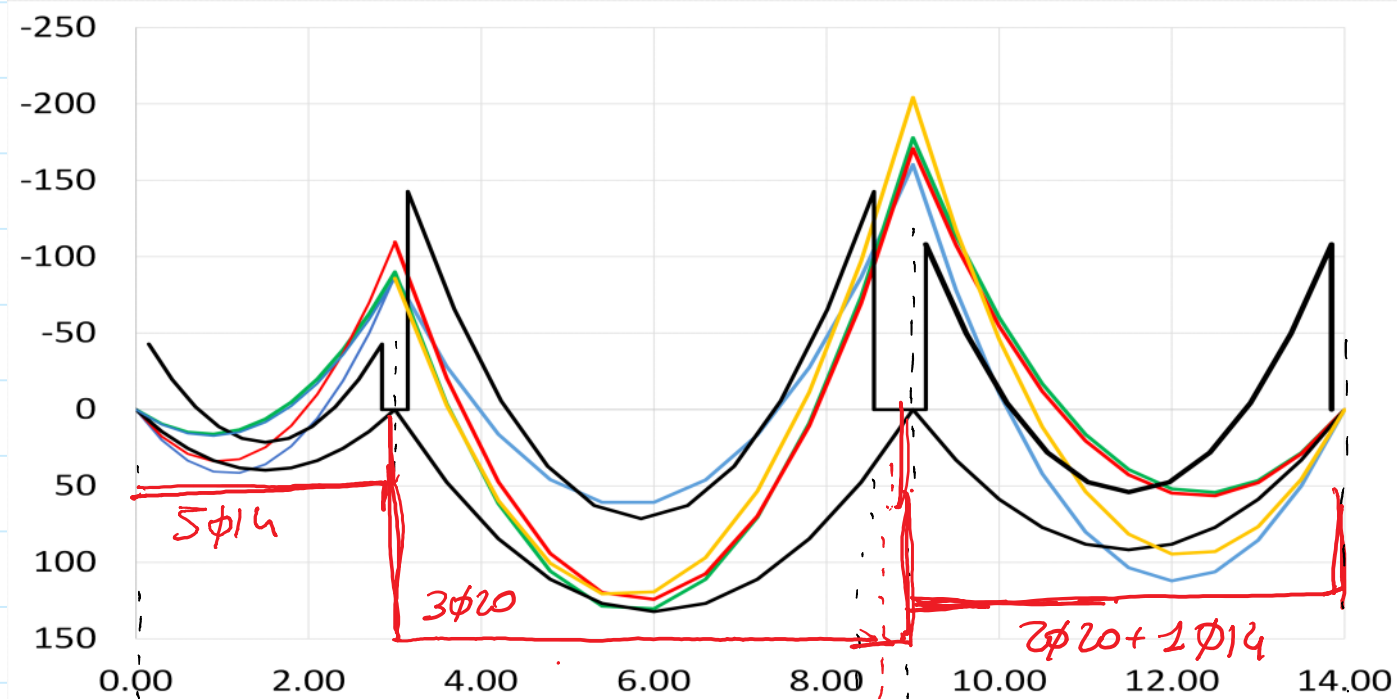
TRAVE EMERGENTE

$$M_{rd} (1\phi 20) = 3.14 \text{ cm}^2 \times 0.9 \times 0.45 \text{ m} \times \frac{391.3 \text{ N}}{\text{mm}^2} \frac{1}{20} = 49.76 \text{ kNm}$$

$$\text{CAMPATA 2-3} \quad 3\phi 20 \rightarrow 3 \times 49.76 \simeq 150 \text{ kNm}$$

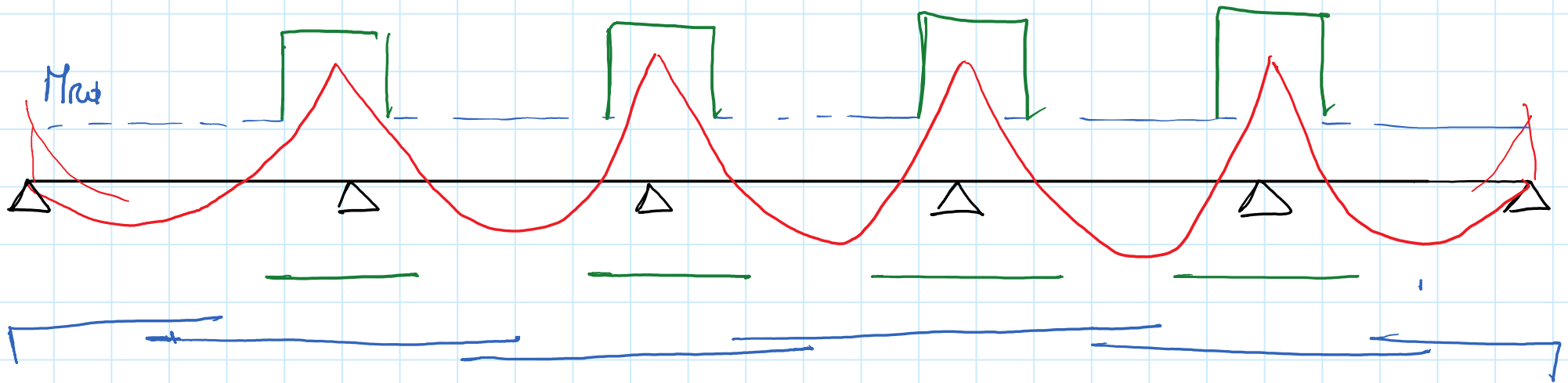
$$3-4 \quad 2\phi 20 + 1\phi 14 \rightarrow 2.5 \times 49.76 \simeq 124 \text{ kNm}$$

# DIAGRAMMA M<sub>rd</sub>



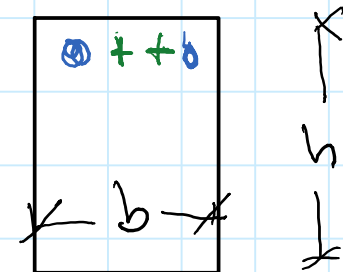
# ARMATURA SUPERIORE — METODO 1

- DISPONGO DA META' CAMPATA A META' CAMPATA  
REGGI-STAFFA PER PORTARE  $\approx 0,40 \div 0,50 M_{max}$
- AGGIUNGO MONCONI PER PORTARE  
INCREMENTO DI MOMENTO



— = REGGI STAFFA

— = MONCONI



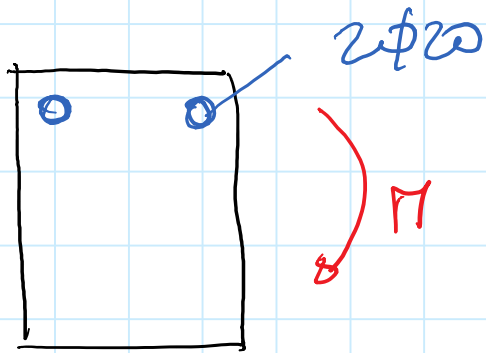
NEL NOSTRO CASO

TRAVE EMERGENTE

$$M_{max} \approx 204 \text{ kNm}$$

$$A_s = \frac{0,40 \times 204 \text{ kNm}}{0,9 \times 0,45 \text{ m} \times 391,3 \text{ N/mm}^2} \times 10 = 5,15 \text{ cm}^2$$

$\Rightarrow 2\phi 20$



+ MONCONI

MONCONI NECESSARI APPOGGIO 3

$$A_s = \frac{204 \text{ kNm}}{0,9 \times 0,45 \text{ m} \times 391,3 \text{ N/mm}^2} \times 10 = 12,37 \text{ cm}^2$$

$\rightarrow 4\phi 20$

$\Rightarrow$  MONCONI AGGIUNTIVI =  $2\phi 20$

# TRAVE A SPESSORE

ARMATURA NECESSARIA AI DUE ESTREMI

$$A_s^{①} \approx \frac{50 \text{ kNm} \cdot \textcolor{red}{\times 10}}{0,9 \times 0,19 \text{ m} \times 391,3} = 7,67 \text{ cm}^2$$

4  $\phi 20$   
oppure  
2  $\phi 20$  + 2  $\phi 14$

$$A_s^{②} \approx \frac{110 \text{ kNm} \cdot \textcolor{red}{\times 10}}{0,9 \times 0,19 \times 391,3} = 16,66 \text{ cm}^2$$

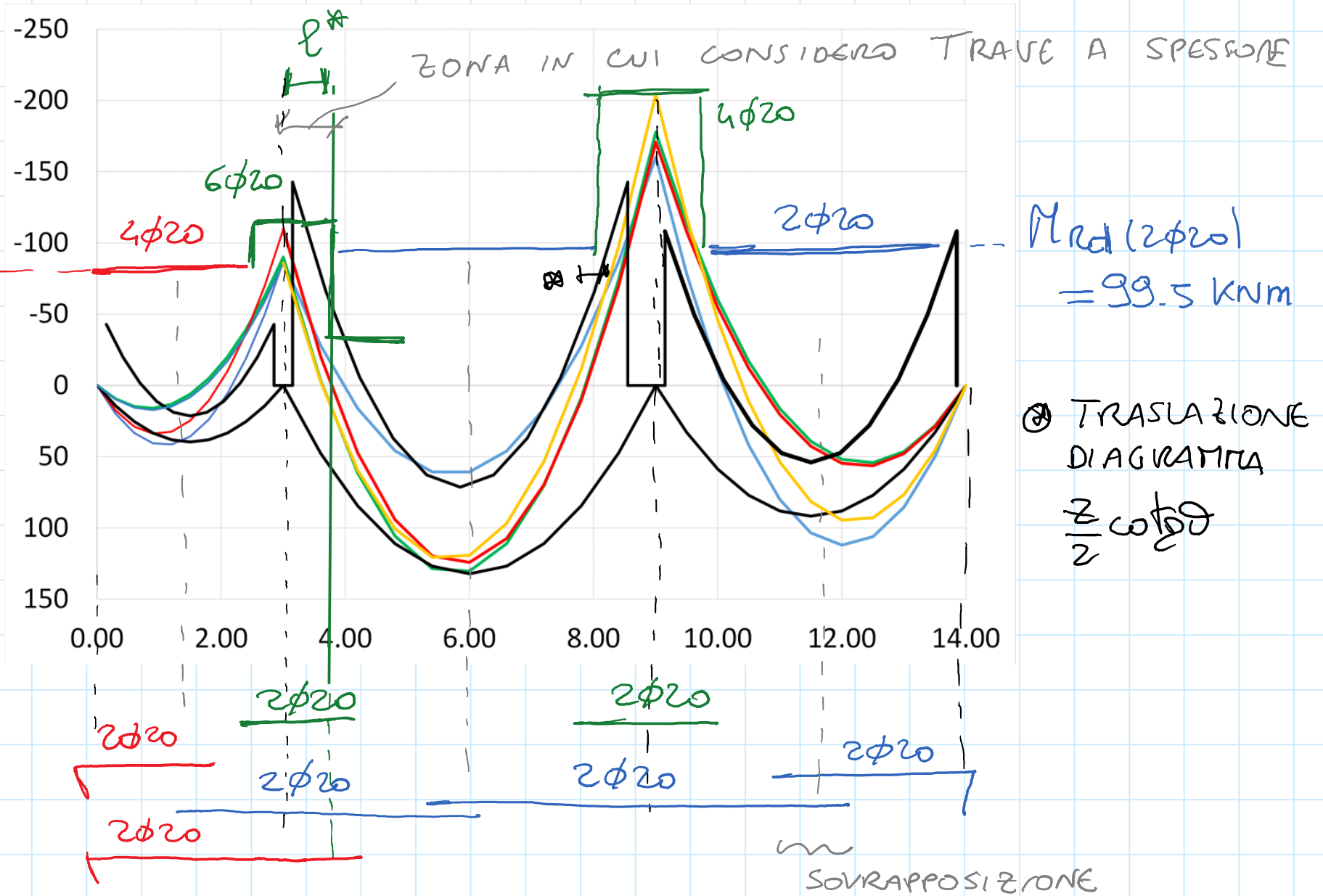
6  $\phi 20$

$$M_{red}^{4\phi 20} = 8 \times M_{red}^{\phi 14} = 88,4 \text{ kNm}$$

$$M_{red}^{6\phi 20} = 132,6 \text{ kNm}$$

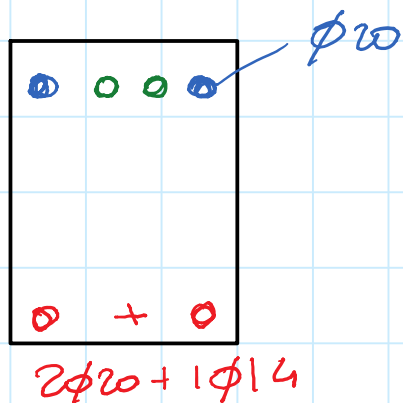
# ARMATURE SUPERIORI E $M_{rd}$

$4\phi 20$   
 $M_{rd} =$   
 $88.4 \text{ kNm}$   
 $(d = 0.13)$

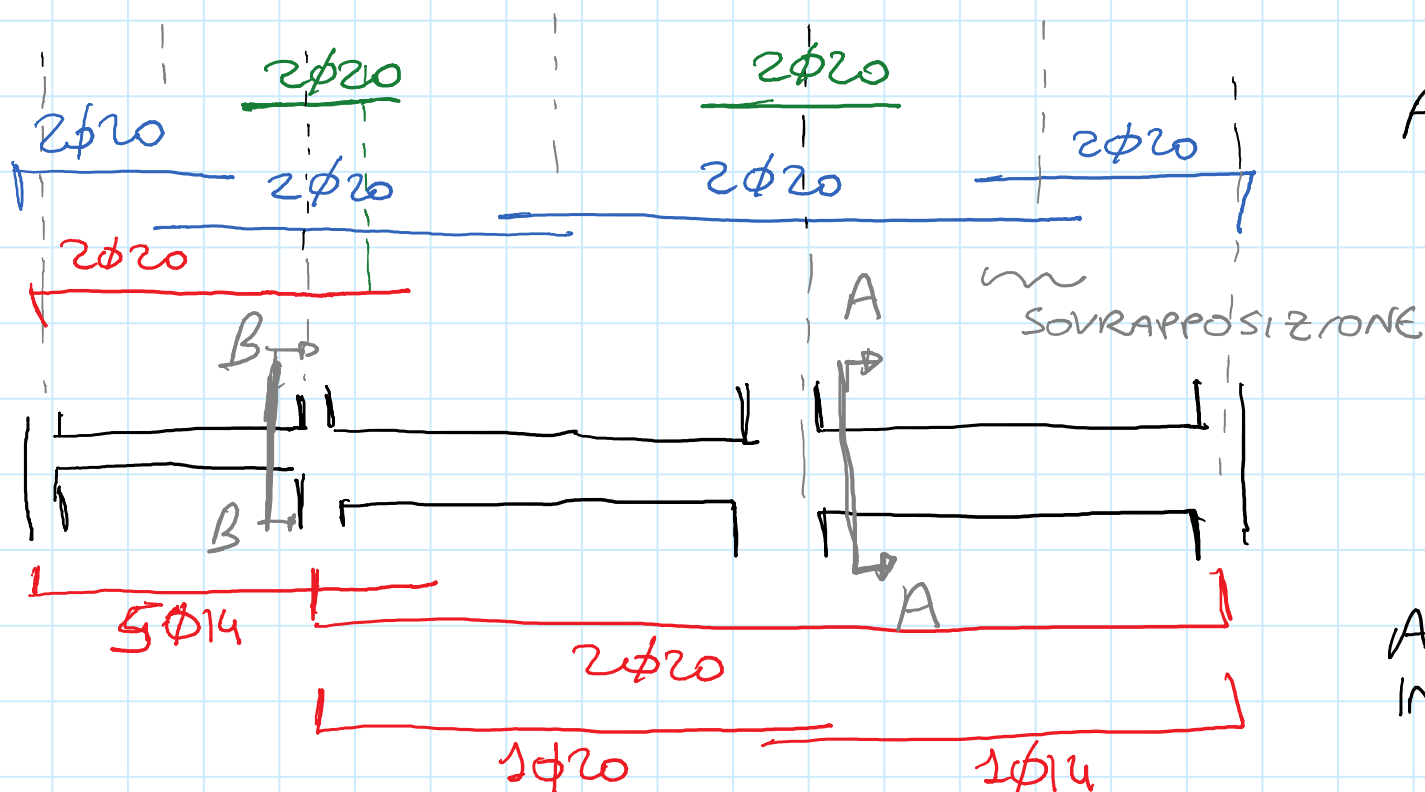
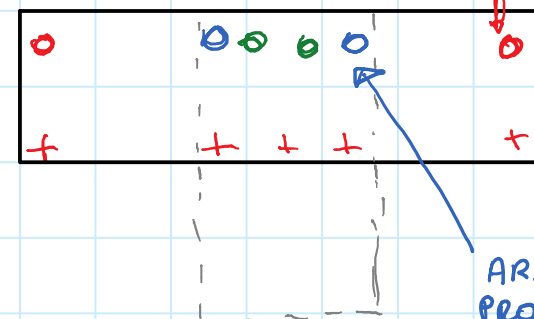


# SEZIONI TRASVERSALI

SEZIONE  
A-A

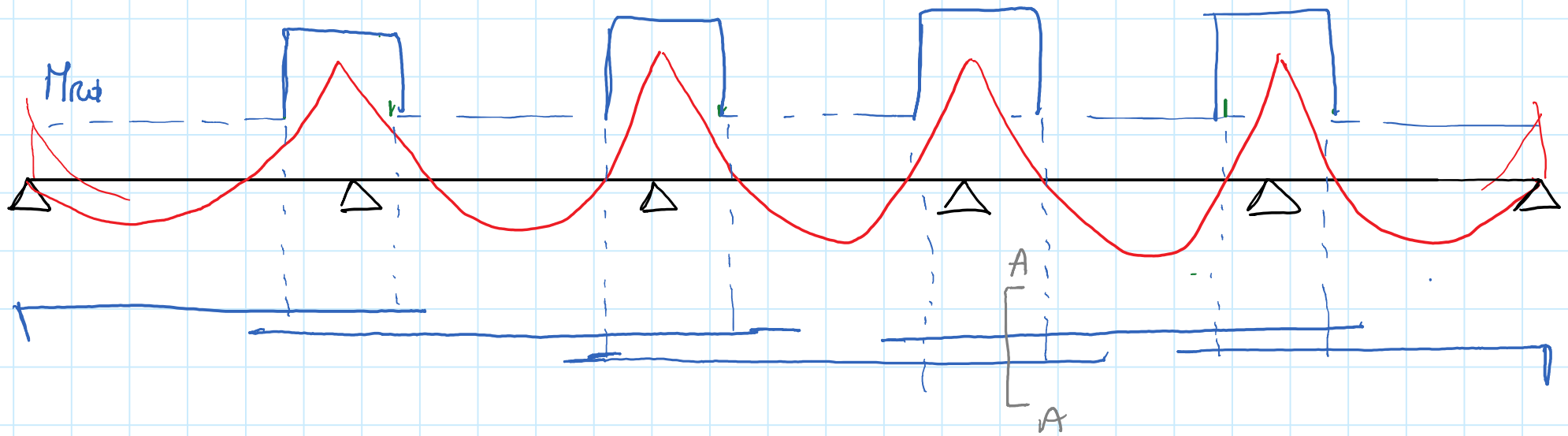


SEZIONE  
B-B

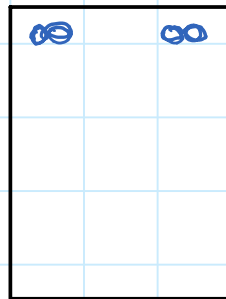


## ARMATURA SUPERIORE — METODO 2

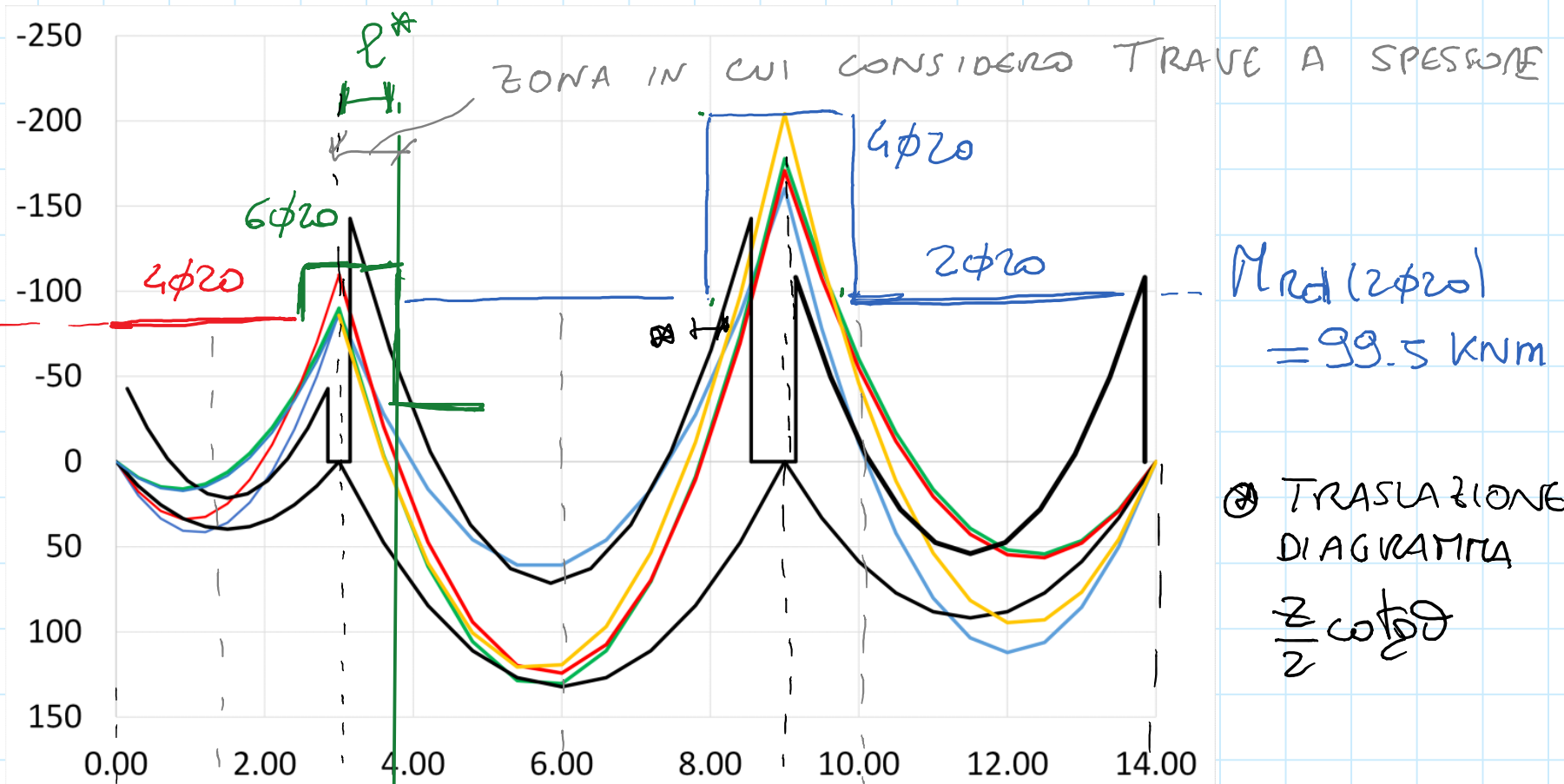
- DISPONGO L'ARMATURA MINIMA SU TUTTA LA CAMPATA E SFRUTTO LA SOVRAPPOSIZIONE DELLE ARMATURE AGU APPOGGI
- SI RIDUCONO I LONCONI NECESSARI



SEZIONE TIPO



# DISPOSIZIONE DELLE ARMATURE SUPERIORI (METODO 2)



4φ20  
 $M_{red} =$   
 88.4 kNm  
 (d = 0.13)

$M_{red}(2\phi20)$   
 = 99.5 kNm

# PROGETTO ARMATURA A TAGLIO

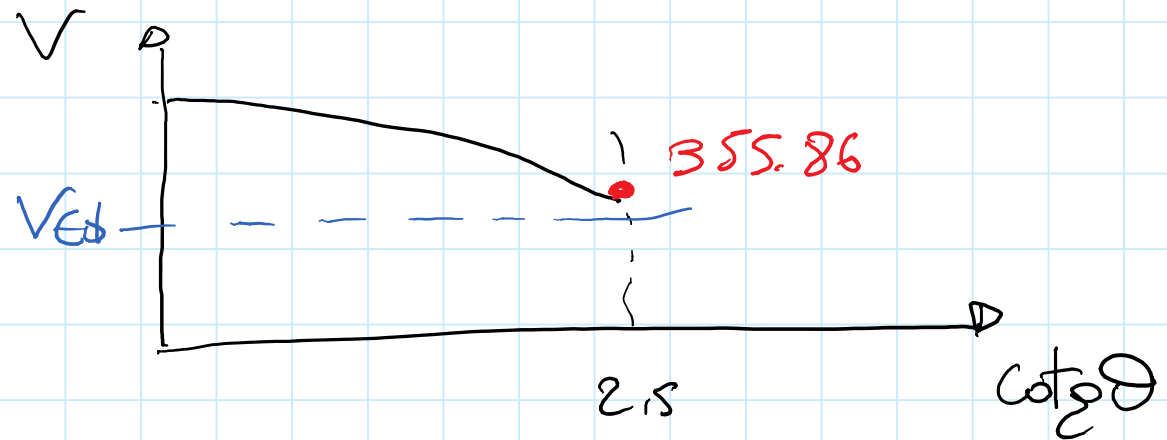
1. CALCOLO  $V_{rd,max} = \sqrt{f_{cd} b z} \frac{\cot \theta}{1 + \cot^2 \theta}$

TRAVE EMERGENTE

$$V_{Ed,max} = 200 \text{ kN}$$

SEZIONE  $30 \times 50$

$$z = 0,9 d \Rightarrow$$



$$V_{rd,max} = \frac{1}{10} 0,5 \times 17 \text{ MPa} \times 30 \text{ cm} \times 0,9 \times 45 \text{ cm} \times \frac{2,5}{1 + 2,5^2}$$
$$1032 \text{ kN} \times \frac{2,5}{1 + 2,5^2} = 355,86 \text{ kN}$$

$$V_{rd,max} > V_{Ed} \quad \forall \cot \theta : 1 \leq \cot \theta \leq 2,5$$

2. DETERMINO  $n_b \frac{A_{st}}{s}$  con  $\cot \theta = 2$

$$V_{red} = n_b \frac{A_{st}}{s} \cdot z \cdot f_{yd} \cot \theta \geq V_{ed}$$

Uso  $\phi 8$  con  $n_b = 2$  ( $A_{\phi 8} = 0,5 \text{ cm}^2$ )

$$\frac{1}{10} \frac{2 \times 0,50 \text{ cm}^2}{s} \cdot 0,9 \times 45 \text{ cm} \times 391,3 \frac{\text{N}}{\text{mm}^2} \cdot 2 \geq 200 \text{ kN}$$

$$\frac{\text{kN} \cdot \text{cm}}{s} \geq \text{kN} \Rightarrow s \text{ in cm} \rightarrow s \leq 15,84 \text{ cm}$$

$$\text{LIMITI DI NORMATIVA: } \left. \begin{array}{l} \frac{A_{st} n_b}{s} \geq 0,25 b w \frac{\text{cm}^2}{\text{m}} \\ s \leq 0,8d \\ s \leq 33 \text{ cm} \end{array} \right\} \phi 8/20$$

DOVE HO  $V_{ed, max} \Rightarrow \phi 8/15$

NOTA 1: POICHE'  $S < 15.85$  POTREI RICALCOLARE  
cotpo:  $V_{rd,s} = V_{ed}$

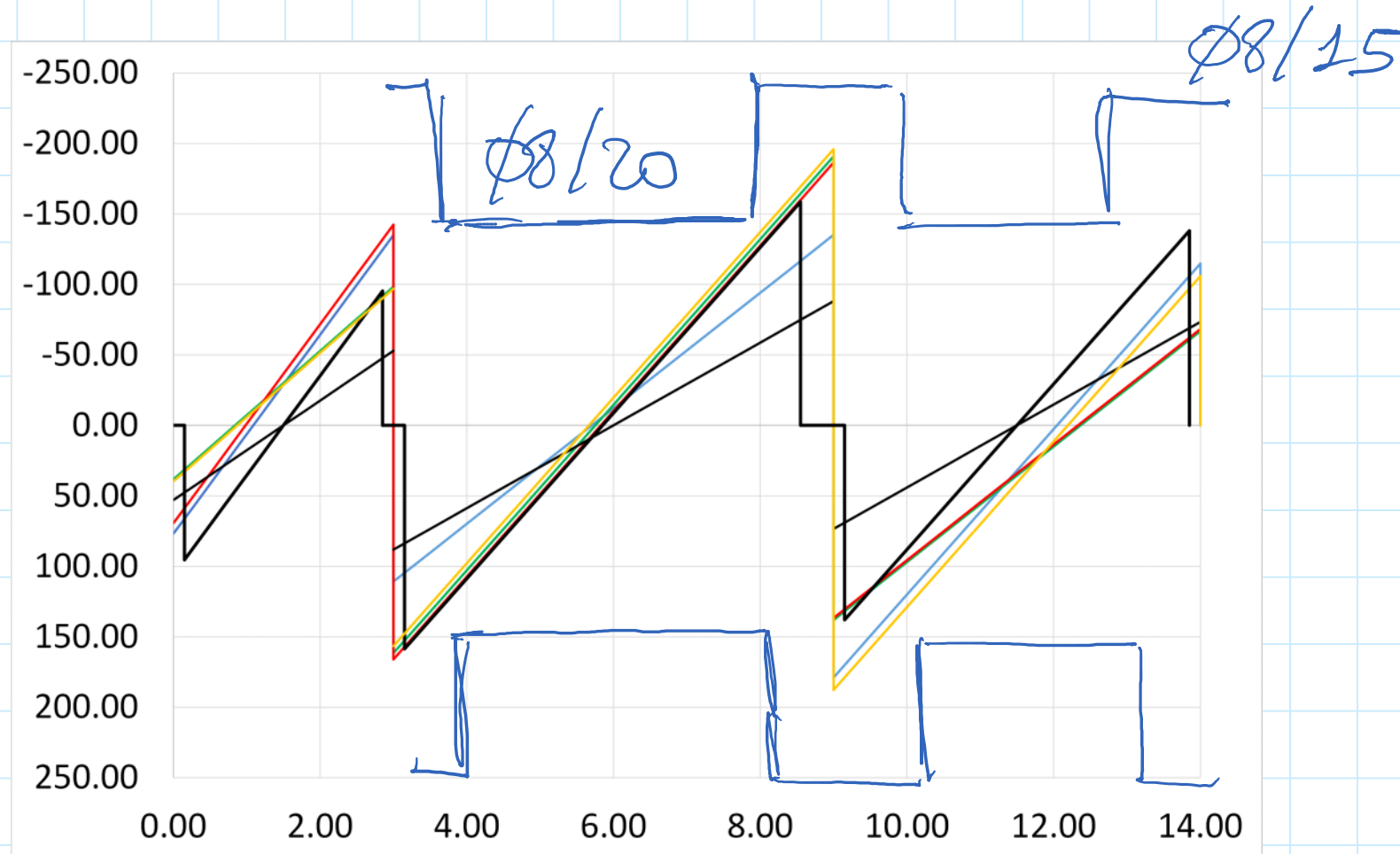
NOTA 2:  $V_{ed, max}$  PUO' ESSERE VALUTATO A  
DISTANZA  $d$  DALL' APPOGGIO

SCELTA FINALE:

IN CAMPATA DISPONGO  $A_{st}/s$  SECONDO MINIMI  
DI NORMATIVA  $\Rightarrow \phi 8/20$

$$V_{rd,s} = \frac{1}{10} \frac{2 \times 0,5 \text{ cm}^2}{20 \text{ cm}} \times 0,8 \times 45 \text{ cm} \times 381,3 \frac{\text{N}}{\text{mm}^2} \times \cotpo^2$$
$$= 258,5 \text{ kN}$$

DOVE NECESSARIO RAFFITTISSO CON  $\phi 8/15$   
(BUONA NORMA RAFFITTISSO AGU ESTREMI  
INDIPENDENTEMENTE DA  $V_{ed}$ )

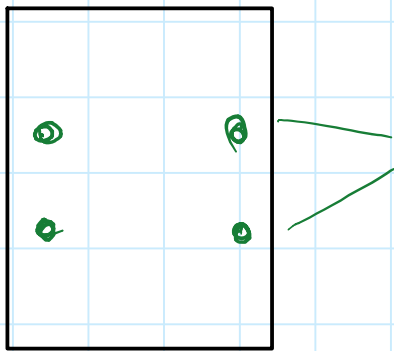


# ARMATURA DI PARETE O TRASLAZIONE DIAGRAMMA MOMENTI

$$A_{s,p} \cdot f_{yd} \geq \frac{V}{2} \cot \rho \theta \Rightarrow$$

$$A_{s,p} = \frac{200 \text{ kN}}{2} \cdot \frac{2}{391,3 \text{ N/mm}^2} \times 10 = 5,1 \text{ cm}^2$$

Se uso  $\phi 14 \Rightarrow A = 1,54 \Rightarrow 3,3 \text{ BARRE} \Rightarrow 4$



$2+2\phi 14$

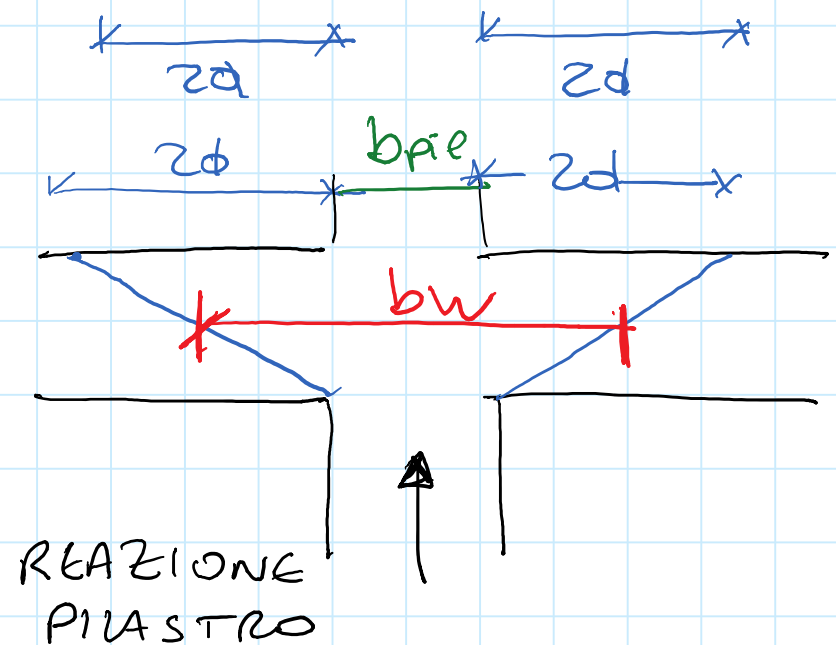
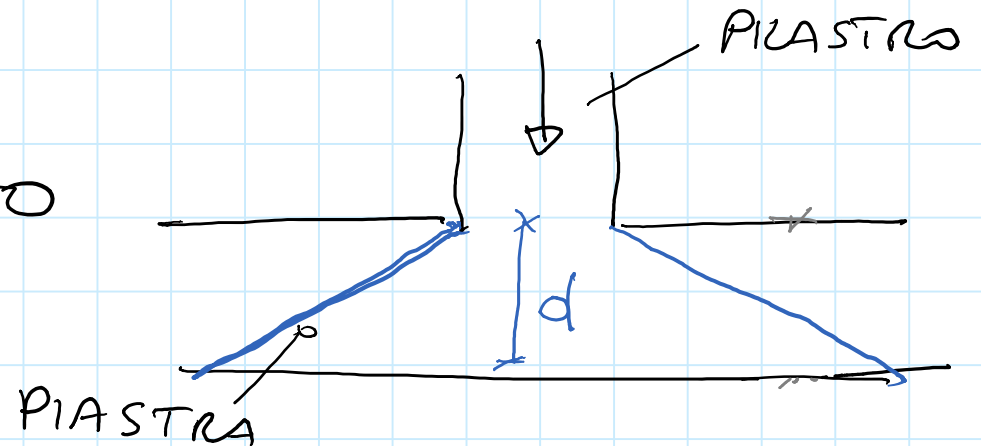
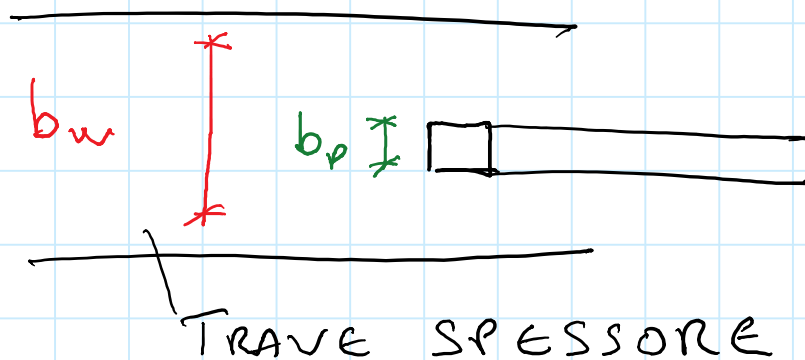
SE DISPONGO  
SOLO  $2\phi 14 \Rightarrow$   
DEVO TRASLARE IL  
DIAGRAMMA DEI  
MOMENTI

# TRAVE A SPESSORE

NON TUTTA LA TRAVE A SPESSORE REAGISCE  
A TAGLIO

FENOMENO PUNZONAMENTO

ANALOGAMENTE PER TRAVE  
A SPESSORE



$$b_w = b_{pie} + 2d$$

(ARMATURA A TAGLIO DA)  
DISPORRE ENTRO  $b_w$

$$b_w = 30 \text{ cm} + 2 \times 19 = 68 \text{ cm}$$

$$V_{rd, max} = \frac{1}{10} 0,5 \times 17 \frac{\text{N}}{\text{mm}^2} \times 68 \text{ cm} \times 0,9 \times 19 \text{ cm} \times \frac{\cot \theta}{1 + \cot^2 \theta}$$

$$\text{se } \cot \theta = 2,5 \Rightarrow V_{rd, max} = 340,82 \text{ kN} \\ > V_{Ed} = 155 \text{ kN}$$

## ARMATURE NECESSARIE

$$n_b \cdot \frac{A_{st}}{s} \cdot f_{yd} \cot \vartheta = 155 \text{ kN}$$

$$n_b = 4 \quad A_{st} = 0,5 \text{ cm}^2 \Rightarrow$$

$$\frac{1}{10} \frac{4 \times 0,5 \text{ cm}^2}{s} \times 0,9 \times 19 \text{ cm} \times 391,3 \frac{\text{N}}{\text{mm}^2} \times 2 = 155 \text{ kN} \Rightarrow$$

$$s \leq 17,3 \text{ cm}$$

## ALTRI LIMITI DI NORMATIVA

$$- n_b \frac{A_{st}}{s} \geq 0,15 b_w = 0,15 \times 68 \frac{\text{cm}^2}{\text{m}} \quad n_b A_{st} = 2 \text{ cm}^2 \Rightarrow$$

$$s \leq 19,6 \text{ cm}$$

$$- s \leq 0,8d = 0,8 \times 19 \text{ cm} \Rightarrow s \leq 15,2 \text{ cm}$$

$\varnothing 8/15$  (4 BRACCI)