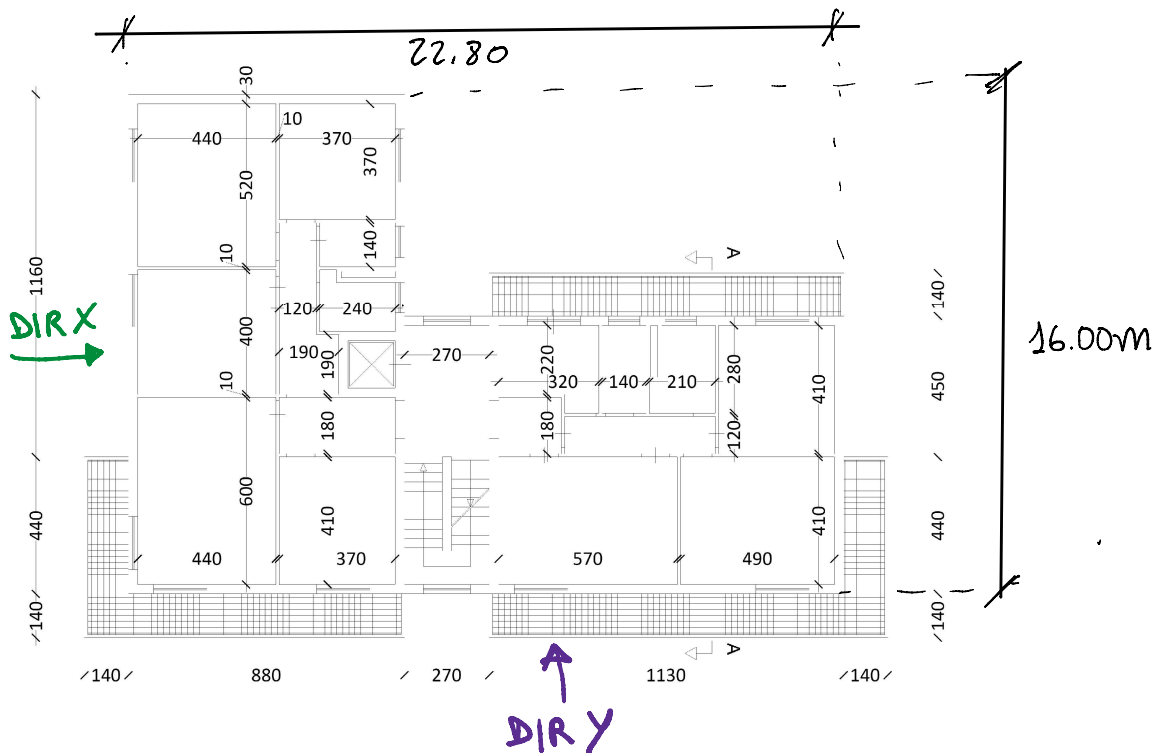


giovedì 2 aprile 2020 13:57

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EDIFICIO A 6 PIANI : $h = 3,3m$

- EDIFICIO A 6 PIANI : $h = 3,5 \text{ m}$
 - PRESSIONE CINETICA DI RIFERIMENTO $q_r = 0,526 \text{ kN/m}^2$
 - CATEGORIA DI ESPOSIZIONE IV

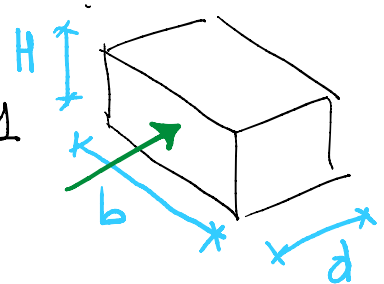


$$H_{TOT} = 6 \times 3,3 \text{ m} = 19,8 \text{ m}$$

VENTO IN DIREZIONE X

$$\begin{aligned} b &= 16.00 \text{ m} \\ d &= 22.80 \text{ m} \\ H &= 19.80 \text{ m} \end{aligned}$$

$$\rightarrow \frac{H}{d} = \frac{19,80}{22,80} = 0.87 \leq 1$$



COEFF. DI PRESSIONE

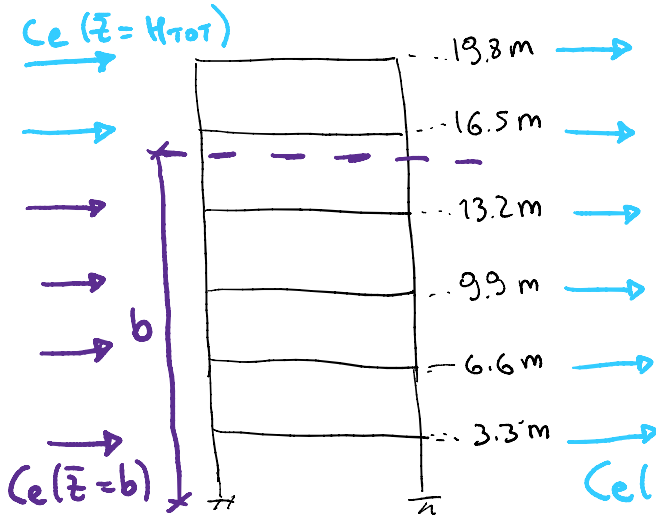
$$C_{pe}(\text{sopravento}) = 0.7 + 0.1 \frac{H}{d} = 0.787$$

$$C_{pe}(\text{SOTTOVENTO}) = -0.3 - 0.2 \frac{H}{d} = -0.474$$

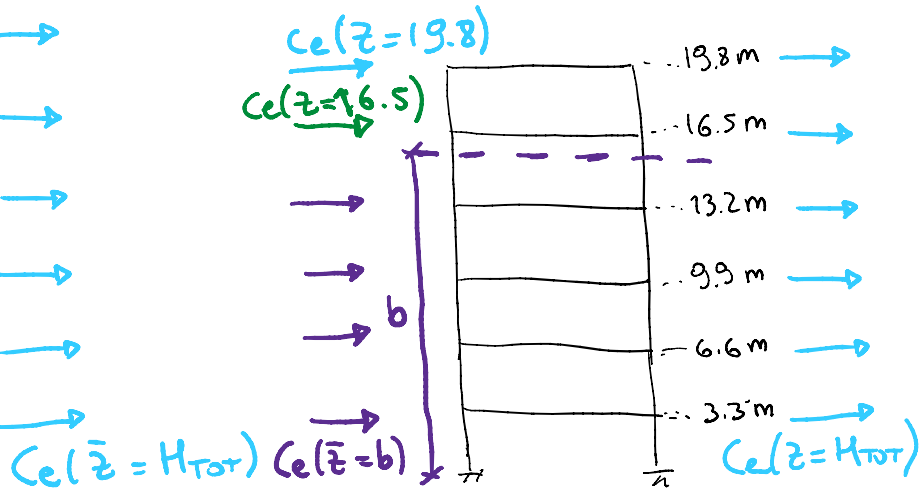
COEFF. DI ESPOSIZIONE

$$\frac{H}{b} = \frac{19,80}{16} > 2 \Rightarrow \text{EDIFICIO "ALTO"}$$

OPZIONE 1



OPZIONE 2



$$P_n = q_r C_d C_p \cdot C_e = q_r C_d \left[\overset{\text{SOPRAV.}}{C_{pe}} \overset{\text{SOPRAV.}}{C_e} - \overset{\text{SOTTOV.}}{C_{pe}} \overset{\text{SOTTOV.}}{C_e} \right]$$

↓
1

| Piano | z imp | q _r [kN/m ²] | c _p soprav | z _e soprav | c _e soprav | c _p sotto | z _e sotto | c _e sotto | p _k [kN/m ²] |
|-------|-------|-------------------------------------|-----------------------|-----------------------|-----------------------|----------------------|----------------------|----------------------|-------------------------------------|
| 6 | 19.8 | 0.526 | 0.787 | 19.8 | 2.27 | -0.474 | 19.8 | 2.27 | 1.504 |
| 5 | 16.5 | 0.526 | 0.787 | 16.5 | 2.13 | -0.474 | 19.8 | 2.27 | 1.449 |
| 4 | 13.2 | 0.526 | 0.787 | 16.0 | 2.11 | -0.474 | 19.8 | 2.27 | 1.439 |
| 3 | 9.9 | 0.526 | 0.787 | 16.0 | 2.11 | -0.474 | 19.8 | 2.27 | 1.439 |
| 2 | 6.6 | 0.526 | 0.787 | 16.0 | 2.11 | -0.474 | 19.8 | 2.27 | 1.439 |
| 1 | 3.3 | 0.526 | 0.787 | 16.0 | 2.11 | -0.474 | 19.8 | 2.27 | 1.439 |

VENTO IN DIREZIONE Y

COEFF. DI PRESSIONE

$$b = 22,80 \text{ m}$$

$$d = 16,00 \text{ m}$$

$$H = 19,80 \text{ m}$$

$$\rightarrow \frac{H}{d} = \frac{19,80}{16,00} = 1,24 > 1$$

$$C_{pe} (\text{SOPRAVENTO}) = 0,8$$

$$C_{pe} (\text{SOTTOVENTO}) = -0,5 - 0,05 \left(\frac{H}{d} - 1 \right) = -0,512$$

COEFF. DI ESPOSIZIONE

$$\frac{H}{b} = \frac{19,80}{22,80} < 1 \Rightarrow \text{EDIFICIO "BASSO"}$$

$C_e (z_e = H_{TOT})$ PER PARETI SOPRAVENTO E SOTTOVENTO

$$\begin{aligned} P_n &= q_r C_d C_p \cdot C_e = q_r C_d \left[\overset{\text{SOPRAV.}}{C_{pe}} \overset{\text{SOPRAV.}}{C_e} - \overset{\text{SOTTOV.}}{C_{pe}} \overset{\text{SOTTOV.}}{C_e} \right] \\ &= 0,526 \frac{\text{KN}}{\text{m}^2} \times 1 \times \left[0,8 \times 2,27 + 0,512 \times 2,27 \right] = \\ &= 1,565 \text{ KN/m}^2 \end{aligned}$$

STATI LIMITE

giovedì 2 aprile 2020 14:39

SLU \rightarrow RESISTENZA DELLA STRUTTURA
+ PROGETTO
(III STADIO)

CARICHI
 $G_d + Q_d + \sum \psi_{0i} Q_{di}$

RESISTENZE
 f_{cd}, f_{yd}

SLE \rightarrow STATO LIMITE DI DEFORMAZIONE

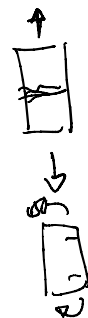
\rightarrow STATO LIMITE DI FESSURAZIONE

\rightarrow STATO LIMITE DI TENSIONI IN ESERCIZIO

STATO LIMITE DI FESSURAZIONE

AMPIEZZA FESSURA
INNESCO FESSURAZIONE

I STADIO DI COMPORTAMENTO


$$\sigma_c^+ \leq f_{ctk}$$
$$\sigma_c^+ \leq f_{ctfk}$$

STATO LIMITE DI TENSIONI DI ESERCIZIO

TENSIONI ELEVATE
(CARICHI DI LUNGA DURATA) \Rightarrow ECCESSIVE
DEF. VISCOSE

COMB. CARICO QUASI PERM.
 $G_k + Q_k \psi_2$

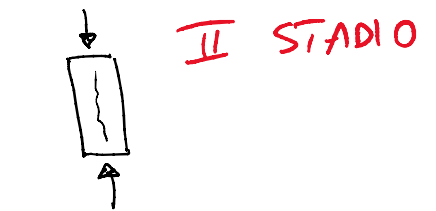
$\sigma_c \leq 0.45 f_{ck}$
II STADIO DI
COMPORTAMENTO

EVITARE FESSURAZIONE
PER FORTE COMPRESSIONE

COMB CARICO RARA

$$G_k + Q_k + \sum \psi_0 Q_{ki}$$

COMB CARICO RARA



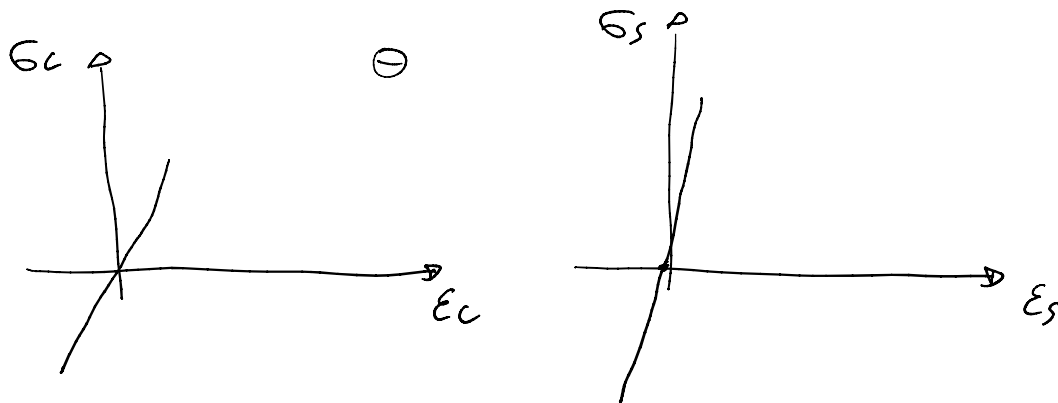
$$\sigma_c \leq 0.60 f_{ck}$$

$$\sigma_s \leq 0.8 f_{yk}$$

SFORZO NORMALE I STADIO

giovedì 2 aprile 2020 14:52

LEGAMI COSTITUTIVI



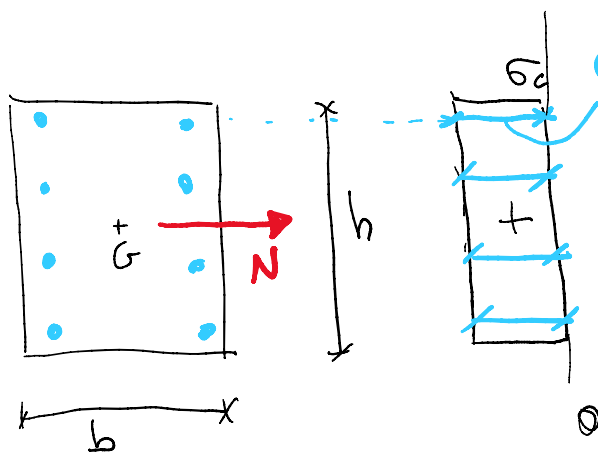
$$N \neq 0; \quad M_x = 0 \quad M_y = 0 \quad \Rightarrow$$

$$\varepsilon_c = \frac{N}{EA}$$

$$\chi_x = -\frac{M_y}{I_y E}$$

$$\chi_y = \frac{M_x}{I_x E}$$

$$\Rightarrow \varepsilon = \varepsilon_c$$



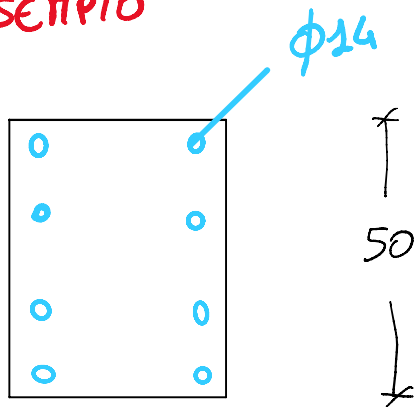
AREA CLS IDEALE

$$A_{ci} = bh + n A_{st}$$

$$\sigma_c = \frac{N}{A_{ci}}$$

$$\sigma_s = n \sigma_c \quad n = \frac{E_s}{E_c}$$

ESEMPIO



$$A_s = 8 \phi 14$$

C 25/30

B 450C

$$N = 200 \text{ kN} \quad (\text{TRAZIONE})$$

$$\eta = \frac{200'000 \text{ MPa}}{31500 \text{ MPa}} = 6,35$$

$$A_{ci} = b h + n \times 8 \times A_{\phi 14}$$

$$A_{\phi 14} = \frac{\pi \cdot \phi^2}{4} = \frac{3,14 \cdot 14^2}{4} = 1,54 \text{ cm}^2$$

$$A_{ci} = 30 \times 50 + 6,35 \times 8 \times 1,54 = 1578 \text{ cm}^2$$

$$\sigma_c^+ = \frac{N}{A_{ci}} = \frac{200 \text{ kN}}{1578 \text{ cm}^2} \times \frac{10^3}{10^2} = 1,27 \text{ MPa}$$

CLS FESSURATO? se $\sigma_c^+ > f_{ctk}$

$$f_{ctk} = 0,7 f_{ctm} = 0,7 \cdot 0,3 \sqrt[3]{f_{ck}^2} = 0,7 \times 0,3 \sqrt[3]{25^2} = 1,80 \text{ MPa}$$

NO CLS ANCORA REAGENTE

QUAL È $N_{\text{FESSURAZIONE}}$?

$$\text{PONGO } \sigma_c^+ = f_{ctk} \rightarrow f_{ctk} = \frac{N_{\text{fess}}}{A_{ci}}$$

$$N_{\text{fess}} = A_{ci} \times f_{ctk}$$

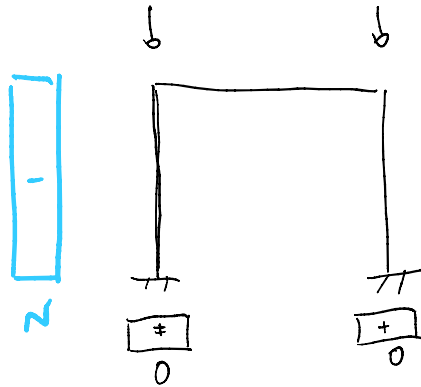
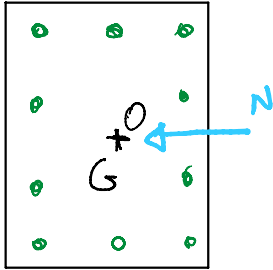
$$N_{\text{fess}} = 1578 \text{ cm}^2 \times 1,80 \frac{\text{N}}{\text{mm}^2} \cdot \frac{10^2}{10^3} = 284 \text{ kN}$$

TENSIONI NEU' ARMATURA PROVOCATE DA N_{less}

$$\sigma_s = n \times \underbrace{\sigma_c}_{p_{ctk}} = 6,35 \times 1,80 \text{ MPa} = 11,43 \text{ MPa}$$

NOTA

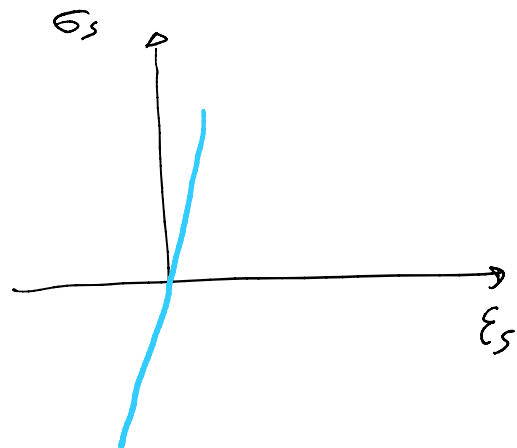
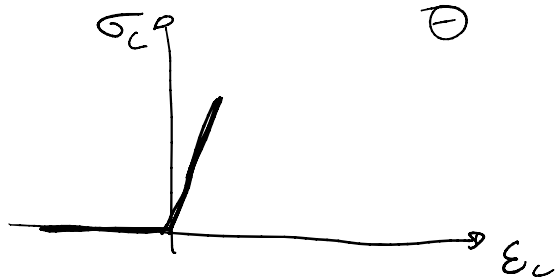
NEL MODELLO DI CALCOLO SONO UTILIZZATE LE
SEZ. GEOMETRICHE DI CLS $\Rightarrow N$ E' APPLICATO IN O.
PER AVERE N CENTRATO DEVO AVERE ARMATURE
SIMMETRICHE $\Rightarrow G \equiv O$



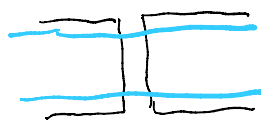
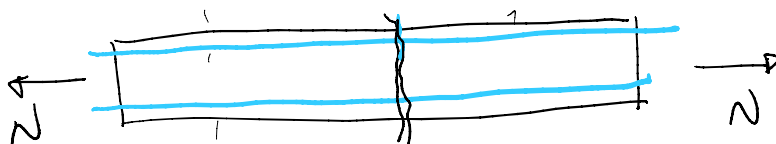
SFORZO NORMALE II STADIO

giovedì 2 aprile 2020 15:19

$N > 0$ TRAZIONE

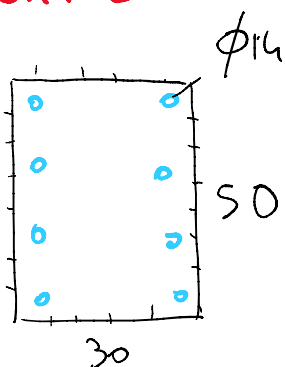


$$\sigma^T \leq p_{ctk} \text{ I STADIO}$$



$$\sigma_c = 0, \quad \sigma_s = \frac{N}{A_{s,tot}}$$

ESEMPIO



$$N = 284 \text{ kN}$$

$$\sigma_s = \frac{N}{A_{s,tot}} = \frac{284 \text{ kN}}{8 \times 1,54 \text{ cm}^2} \times 10$$

$$= 230,5 \text{ MPa}$$

$$\sigma_s \leq 0,8 f_{yk}$$

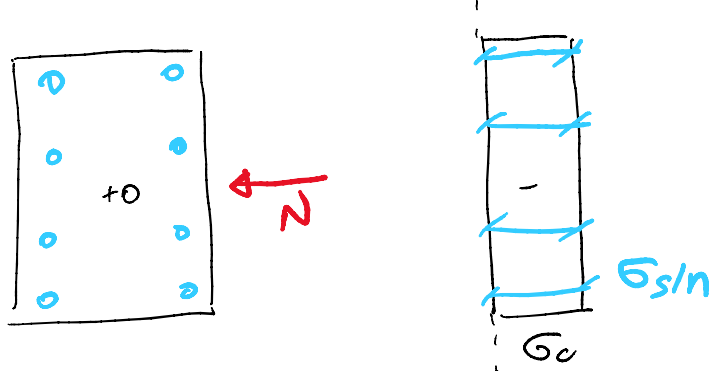
PER LIMITARE
AMPIEZZA
FESSURE

SFORZO NORMALE II STADIO

giovedì 2 aprile 2020

15:34

$N < 0$ COMPRESSIONE



$$A_{ci} = \underbrace{A_c}_{bh} + n A_{s, tot}$$

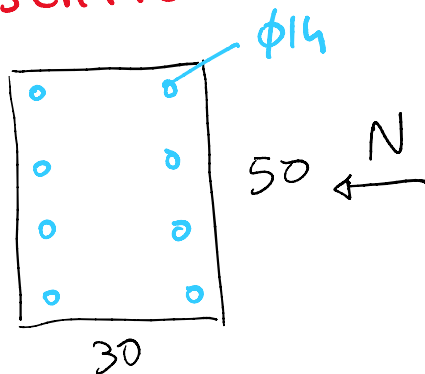
$$\sigma_c = \frac{N}{A_{ci}}$$

$$\sigma_s = n \cdot \sigma_c$$

CARICHI DI LUNGA DURATA

$$\Rightarrow n = 15$$

ESEMPIO



CARICHI LUNGA DURATA

B450C

C25/30

$$N = -2000 \text{ kN}$$

$$\sigma_c = ? \quad \sigma_s = ?$$

$$A_{ci} = bh + n \cdot A_{s, tot} = 30 \times 50 + 15 \cdot 8 \times 1,54 \text{ cm}^2 = 1684 \text{ cm}^2$$

$$\sigma_c = \frac{-2000 \text{ kN}}{1684 \text{ cm}^2} \times 10 = -11,88 \text{ MPa}$$

Se N OTTENUTO DA COMB. RARA

$$0.6 f_{ck} = 0.6 \times 25 \\ = 15 \text{ MPa}$$

$$11.88 < 15 \Rightarrow \text{SODDISFATTA}$$

Se N OTTENUTO DA COMB. QUASI PERMANENTE

$$0.45 f_{ck} = 0.45 \times 25 \\ = 11.25 \text{ MPa}$$

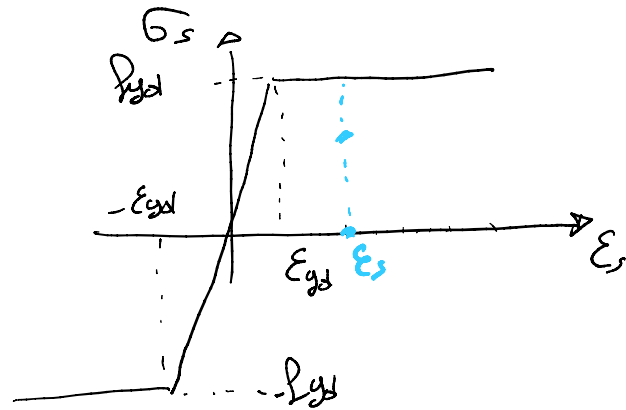
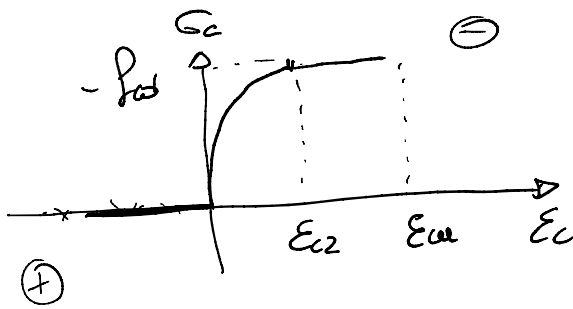
$$11.88 > 11.25 \Rightarrow \text{NON SODDISFATTA}$$

$$\sigma_s = n \sigma_c = -15 \times 11.88 \text{ MPa} = -178.2 \text{ MPa}$$

SFORZO NORMALE III STADIO

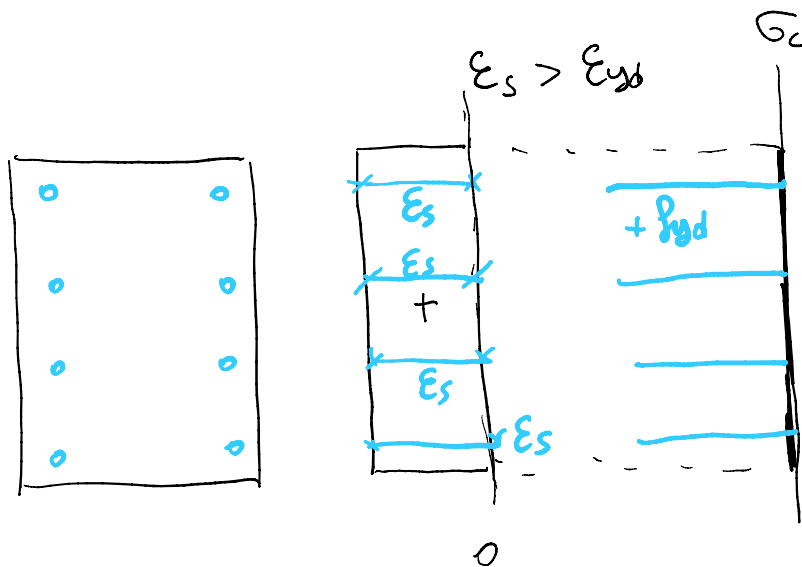
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$$N_{Ed} > 0$$



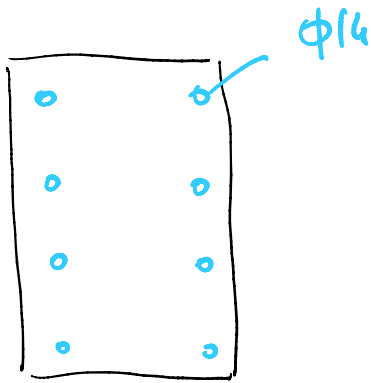
CALCOLARE N_{Rd}^+

- PARTO DA DIAGRAMMA ϵ LIMITE
- CALCOLO LE σ DAI LEGAMI COSTITUTIVI
- $N = \int \sigma dA$



$$N_{Rd}^+ = \sum A_s \cdot \sigma_s = f_{yd} \cdot A_{s,tot}$$

ESEMPIO



$$N_{Rd} = ?$$

B450C

$$f_{yd} = \frac{f_{yk}}{\gamma_s} = \frac{450 \text{ MPa}}{1.25} = 360 \frac{\text{N}}{\text{mm}^2}$$

$$A_{s, \text{tot}} = 8 \times 1,54 \text{ cm}^2 = 12,32 \text{ cm}^2$$

$$N_{Rd} = 12,32 \text{ cm}^2 \times 360 \frac{\text{N}}{\text{mm}^2} \times \frac{1}{10} =$$

$$= 443,5 \text{ kN}$$